

G. STRUCTURAL - GENERAL

- G1 SCOPE**
THE NOTES ON THIS SHEET ARE GENERAL AND APPLY TO THE "S" SERIES OF STRUCTURAL DRAWINGS.
- G2 APPLICABLE SPECIFICATIONS AND CODES**
CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE 2006 INTERNATIONAL BUILDING CODE WITH LOCAL AMENDMENTS.
- G3 DIMENSIONS**
STRUCTURAL DIMENSIONS CONTROLLED BY OR RELATED TO MECHANICAL AND ELECTRICAL EQUIPMENT SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION.
- G4 PROVISIONS FOR EQUIPMENT**
STRUCTURAL DRAWINGS SHALL BE READ ALONG WITH MECHANICAL, EQUIPMENT, AND ELECTRICAL DRAWINGS TO ENSURE PROPER INTERFACE REGARDING EQUIPMENT SUPPORTS, ANCHORAGES, OPENINGS, PIPE SLEEVES, RECESSES, ETC. PRIOR TO CASTING CONCRETE.
- G5 CONSTRUCTION LOADS**
STRUCTURES HAVE BEEN DESIGNED FOR OPERATIONAL LOADS ON THE COMPLETED STRUCTURES. DURING CONSTRUCTION, STRUCTURES SHALL BE PROTECTED BY BRACING AND SUPPORTS AS NEEDED. CONTRACTOR IS RESPONSIBLE FOR DESIGN AND MAINTENANCE OF SUCH TEMPORARY SUPPORTS.

F. STRUCTURAL DESIGN

- F1 DESIGN CODE**
DESIGN IS IN ACCORDANCE WITH THE 2006 INTERNATIONAL BUILDING CODE AND ASCE 7-05, EXCEPT WHERE OTHER APPLICABLE LOCAL CODES OR THE FOLLOWING NOTES ARE MORE RESTRICTIVE.
- F2 FOUNDATION DESIGN**
DESIGN IS BASED ON TABLE 1804.2 IN THE 2006 IBC.
(1) ALLOWABLE BEARING PRESSURE = 3000 PSF ON NATIVE COMPACTED MATERIAL
(2) LATERAL PASSIVE PRESSURE = 200 PCF
(3) BASE FRICTION COEFFICIENT = 0.35

L. DESIGN LOADS

- A. LIVE**
(1) ROOF = 20 PSF (NON-REDUCIBLE)
- B. SNOW**
(1) IMPORTANCE FACTOR = 1.0
(2) ROOF SNOW LOAD PF = 50 PSF (PER BUILDING DEPT)
- C. WIND**
(1) OCCUPANCY CATEGORY: II
(2) IMPORTANCE FACTOR: 1.0
(3) BASIC WIND SPEED: 105 MPH
(4) WIND EXPOSURE: C
(5) Kz1 = 1.0
- D. SEISMIC**
(1) OCCUPANCY CATEGORY = II
(2) IMPORTANCE FACTOR = 1.0
(3) SITE CLASS: D
(4) Ss = 0.560 S1 = 0.268
(5) SDS = 0.505 SD1 = 0.333
(6) SEISMIC DESIGN CATEGORY: SDC = D
(7) EQUIVALENT LATERAL FORCE
CANTILEVERED COLUMN SYSTEM
R = 1.5 Cs = 0.34

C. CONCRETE

- C1 APPLICABLE CODE**
CONCRETE CONSTRUCTION SHALL CONFORM TO THE 2005 EDITION OF THE ACI BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, ACI-318.
- C2 CONCRETE SPECIFICATIONS**
THE NOTES BELOW SHALL SERVE AS MINIMUM REQUIREMENTS, CONCRETE SHALL CONFORM TO CBJ STANDARD SPECIFICATION FOR STRUCTURAL CONCRETE, UNLESS NOTED OTHERWISE IN PROJECT DOCUMENTS.
- C2.1 DESIGN STRENGTHS**
CAST-IN-PLACE CONCRETE: fc = 4000 psi @ 28 DAYS, 5% AIR ENTRAINMENT. MAX W/C=0.45, 1 1/2" COARSE AGGREGATE, ADD PLASTICIZER AS NEEDED, MAX SLUMP W/O PLASTICIZER SHALL BE 4" AND 8" WITH PLASTICIZER.
- C3 REINFORCING STEEL DETAILS**
DETAILING, FABRICATION AND ERECTION OF REINFORCING STEEL, UNLESS OTHERWISE NOTED, SHALL BE IN ACCORDANCE WITH DETAILS AND DETAILING OF CONCRETE REINFORCEMENT ACI 315.
- C4 CONCRETE COVER**
CONCRETE COVER FOR REINFORCING BARS SHALL CONFORM TO THE REQUIREMENTS OF ACI 318.
- C5 MOISTEN SUBGRADE**
MOISTEN SUBGRADE BEFORE PLACING CONCRETE, WHERE CONCRETE IS NOT PLACED DIRECTLY ON PLASTIC SHEETING.
- C6 SPECIAL WEATHER CONCRETING**
FOR SPECIAL WEATHER CONCRETING (HOT & COLD CONCRETING) SEE REPORTS OF ACI COMMITTEE 305, "HOT WEATHER CONCRETING", AND ACI 306, "COLD WEATHER CONCRETING."
- C7 CONCRETE FINISHES**
CONCRETE FINISHES SHALL CONFORM TO PROJECT SPECIFICATIONS.

- C8 CURING**
APPLY LIQUID MEMBRANE CURING COMPOUND TO HORIZONTAL SURFACES AS SOON AS POSSIBLE AFTER FINISHING, IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS. LIQUID CURING COMPOUNDS SHALL NOT BE APPLIED TO SURFACES WHICH ARE TO RECEIVE CONCRETE TOPPING.
- C9 CONTROL JOINTS**
SLAB CONTROL JOINTS SHALL BE 1/8" WIDE CONTINUOUS SAW CUT AT DEPTH EQUAL TO 1/4 THE SLAB THICKNESS. FILL JOINT WITH ELASTOMERIC JOINT SEALANT. VERTICAL CONTROL JOINTS IN WALLS SHALL BE FORMED WITH 3/4 INCH CHAMFER STRIP AND FILLED WITH ELASTOMERIC SEALANT. THE ELASTOMERIC JOINT SEALANT SHALL CONFORM TO ASTM C920, TYPE S OR M, GRADE NS, CLASS 50.
- C10 CONSTRUCTION JOINTS**
LOCATION OF ANY CONSTRUCTION JOINTS NOT SHOWN ON THE DRAWINGS SHALL HAVE THE APPROVAL OF THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. CONSTRUCTION JOINTS SHALL BE DETAILED AS SHOWN ON THE DRAWINGS. UNLESS A METAL KEED FORM IS USED, ALL CONSTRUCTION JOINTS SHALL BE ROUGHENED TO A MINIMUM 1/4" AMPLITUDE. ALL JOINT SURFACES SHALL BE THOROUGHLY CLEANED TO REMOVE GREASE, LOOSE CONCRETE, AND LAITANCE OR OTHER BOND REDUCING MATERIAL. APPLY BONDING AGENT ASTM C1059 TYPE II PRIOR TO PLACING FRESH CONCRETE.

- C11 ISOLATION AND EXPANSION JOINTS**
PERFORMED EXPANSION JOINT FILLER SHALL BE NONEXTRUDING, RESILIENT BITUMINOUS TYPE CONFORMING TO ASTM D1751. MATERIAL THICKNESS SHALL BE 1/4 INCH FOR ISOLATION JOINTS AND 1/2 INCH FOR EXPANSION JOINTS, UNLESS NOTED OTHERWISE.
- C12 DOWELS**
DOWELS SHALL BE AT LEAST THE SAME SIZE AND SPACING AS BARS WITH WHICH THEY ARE LAPPED. THE DOWEL EMBEDMENT SHALL BE MINIMUM 6" UNLESS NOTED OTHERWISE IN DETAILS.
- C13 BAR SPLICES**
SPLICES OF REINFORCING STEEL BAR SHALL BE AS SHOWN IN TYPICAL DETAILS DRAWINGS AND ACI 318. SPLICES SHALL BE CLASS B UNLESS OTHERWISE NOTED. THE LENGTH OF LAP SPlice OF BARS OF DIFFERENT DIAMETER SHALL BE BASED ON THE SMALLER DIAMETER. BAR SPLICES MAY ALSO BE MADE BY WELDING IN ACCORDANCE WITH AWS SPEC D 1.4 IF APPROVED BY THE REGISTERED DESIGN PROFESSIONAL.

- C14 RESTRICTED BAR ANCHORAGE**
IN CASES WHERE REINFORCING BARS CANNOT BE EXTENDED AS FAR AS REQUIRED DUE TO THE LIMITED EXTENT OF THE ADJACENT CONCRETE STRUCTURE, THE BARS SHALL EXTEND AS FAR AS POSSIBLE AND END IN STANDARD HOOKS.
- C15 STANDARD HOOKS**
BARS ENDING IN RIGHT ANGLE BENDS OR HOOKS SHALL CONFORM TO THE REQUIREMENTS OF ACI 318.

- C16 CHAMFERS**
EXCEPT AS OTHERWISE REQUIRED, EXPOSED CONCRETE CORNERS AND EDGES SHALL HAVE 3/4" CHAMFERS. RE-ENTRANT CORNERS SHALL NOT HAVE FILLETS.
- C17 CAST-IN-PLACE CONCRETE ANCHORS**
ANCHORS SHALL BE HEADED BOLTS OF ASTM F1554 GRADE 36 MATERIAL WITH ASTM A563 HEAVY HEXAGONAL NUTS AND ASTM A36 PLATE WASHERS WITH MINIMUM SIZE CONFORMING TO TABLE 14-2 OF THE CURRENT AISC STEEL CONSTRUCTION MANUAL, UNLESS NOTED OTHERWISE. ALTERNATELY, ANCHORS SHALL BE THREADED AND NUTTED ROD CONFORMING TO ASTM F1554 GRADE 36 OR ASTM A36, WITH THE EMBEDDED NUT THREADED ON AND WELDED TO THE ROD. ALL MATERIALS EXPOSED TO MOISTURE OR WEATHER SHALL BE HOT DIP GALVANIZED UNLESS NOTED OTHERWISE.

- C18 POST-INSTALLED MECHANICAL ANCHORS**
MECHANICAL (EXPANSION, SCREW) ANCHORS AND THEIR PROPERTIES SUCH AS DIAMETER, SPACING, EDGE DISTANCE, EMBEDMENT AND MATERIAL/FINISH SHALL CONFORM TO DETAILS IN THESE DRAWINGS. AT CONTRACTOR'S OPTION, AN EQUIVALENT ALTERNATE MECHANICAL ANCHOR MAY BE SUBSTITUTED, PROVIDED THE ALTERNATE PRODUCT SUBMITTAL IS SUPPLEMENTED WITH CALCULATIONS INDICATING THAT THE PRODUCT MEETS OR EXCEEDS PROPERTIES OF THE ORIGINAL PRODUCT. THE SUPPLEMENTAL CALCULATIONS SHALL BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF PROJECT LOCATION. ACCEPTABLE MECHANICAL ANCHORS SHALL BE ICC APPROVED FOR SEISMIC LOADS AND USE IN CRACKED AND UNCRACKED CONCRETE. SUBMITTAL SHALL INCLUDE PRODUCT ESR REPORT.

- C19 POST-INSTALLED ADHESIVE ANCHORS**
ADHESIVE (CHEMICAL, EPOXY) ANCHORS AND THEIR PROPERTIES SUCH AS DIAMETER, SPACING, EDGE DISTANCE, EMBEDMENT AND MATERIAL/FINISH SHALL CONFORM TO DETAILS IN THESE DRAWINGS. AT CONTRACTOR'S OPTION, AN EQUIVALENT ALTERNATE ADHESIVE ANCHOR MAY BE SUBSTITUTED, PROVIDED THE ALTERNATE PRODUCT SUBMITTAL IS SUPPLEMENTED WITH CALCULATIONS INDICATING THAT THE PRODUCT MEETS OR EXCEEDS PROPERTIES OF THE ORIGINAL PRODUCT. THE SUPPLEMENTAL CALCULATIONS SHALL BE STAMPED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF PROJECT LOCATION. ACCEPTABLE ADHESIVE ANCHORS SHALL BE ICC APPROVED FOR SEISMIC LOADS AND USE IN CRACKED AND UNCRACKED CONCRETE. SUBMITTAL SHALL INCLUDE PRODUCT ESR REPORT.

- C20 INSTALLATION OF POST-INSTALLED ANCHORS**
ALL MECHANICAL AND ADHESIVE ANCHORS SHALL BE INSTALLED IN STRICT CONFORMANCE TO MANUFACTURER'S DIRECTIONS. SEE SPECIAL INSPECTION SECTION FOR NOTES PERTAINING TO REQUIRED SPECIAL INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE.

S. STEEL

- S1 CODES AND SPECIFICATIONS**
STEEL CONSTRUCTION SHALL CONFORM TO THE SPECIFICATIONS AND STANDARDS AS CONTAINED IN THE 13TH EDITION OF THE AISC STEEL CONSTRUCTION MANUAL.
- S2 MATERIAL**
BARS, PLATES, ANGLES, AND CHANNELS SHALL CONFORM TO ASTM A36 W SECTIONS SHALL CONFORM TO ASTM A992 HOLLOW STRUCTURAL SECTIONS SHALL CONFORM TO ASTM A500 GRADE B BOLTS SHALL CONFORM TO ASTM A325 UNLESS NOTED OTHERWISE HEAVY HEXAGONAL NUTS SHALL CONFORM TO ASTM A563 WASHERS SHALL CONFORM TO ASTM F436 UNLESS NOTED OTHERWISE
- S3 WELDING - GENERAL**
WELDING SHALL CONFORM TO AWS D1.1-04 "STRUCTURAL WELDING CODE - STEEL" ELECTRODE SHALL BE E70XX GROUP, LOW HYDROGEN. LIGHT GAUGE STEEL WELDING SHALL CONFORM TO AWS D1.3-98. WELDING SHALL BE CONDUCTED BY WELDERS CERTIFIED BY THE AWS.
- S4 WELDING - SEISMIC FORCE RESISTING SYSTEMS**
IN ADDITIONAL TO THE GENERAL WELDING REQUIREMENTS, THE FOLLOWING REQUIREMENTS APPLY TO WELDS USED IN MEMBERS AND CONNECTIONS WHICH ARE PART OF A SEISMIC LOAD RESISTING SYSTEM. WELDING SHALL CONFORM TO AWS D1.8 "STRUCTURAL WELDING CODE - SEISMIC SUPPLEMENT." WELDING SHALL BE PERFORMED IN ACCORDANCE WITH AISC 341 APPENDIX W. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH A WELDING PROCEDURE SPECIFICATION AS REQUIRED IN AWS D1.1 AND AWS D1.8 AND SHALL BE APPROVED BY THE ENGINEER. WELDS SHALL BE MADE WITH A FILLER METAL THAT CAN PRODUCE WELDS THAT HAVE A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20 FT-LB AT 0 °F, AS DETERMINED BY THE APPROPRIATE AWS CLASSIFICATION TEST METHOD OR MANUFACTURER CERTIFICATION. WELDING ELECTRODES AND ELECTRODE-FLUX COMBINATIONS SHALL MEET THE REQUIREMENTS OF H16 (16ML MAXIMUM DIFFUSIBLE HYDROGEN PER 100 GRAMS DEPOSITED WELD METAL) AS TESTED IN ACCORDANCE WITH AWS A4.3.
- S5 DEMAND CRITICAL WELDS**
WELDS SPECIFIED AS "DEMAND CRITICAL" OR "DC" ON THE DRAWINGS SHALL BE MADE WITH A FILLER METAL CAPABLE OF PROVIDING A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20 FT-LB AT MINUS 20 °F AS DETERMINED APPROPRIATE BY THE AWS CLASSIFICATION TEST METHOD OR MANUFACTURER CERTIFICATION.
- S6 LOWEST ANTICIPATED SERVICE TEMPERATURE**
THE LOWEST ANTICIPATED SERVICE TEMPERATURE (LAST) IS -22 °F.

I. SPECIAL INSPECTION

I1 CODE
SPECIAL INSPECTION SHALL CONFORM TO SECTION 1704 AND 1707 OF THE 2006 INTERNATIONAL BUILDING CODE AND ANY LOCAL AMENDMENTS. SPECIAL INSPECTIONS SHALL BE CARRIED OUT ACCORDING TO THE "STATEMENT OF SPECIAL INSPECTIONS". LABORATORIES FOR MATERIAL TESTING AND/OR AGENCIES FOR TESTING SERVICES SHALL BE SELECTED BY, ENGAGED BY, AND RESPONSIBLE TO THE ARCHITECT. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS FOR EACH INSPECTION TO THE BUILDING OFFICIAL, PROJECT REPRESENTATIVE, CONTRACTOR, AND OWNER. THE SPECIAL INSPECTION AGENCY SHALL SUBMIT A FINAL REPORT INDICATING THE WORK REQUIRING SPECIAL INSPECTION WAS INSPECTED AND IS IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THAT ALL DISCREPANCIES NOTED IN THE INSPECTION REPORTS HAVE BEEN CORRECTED.

THE FOLLOWING ITEMS REQUIRE SPECIAL INSPECTION PER IBC CHAPTER 17. THESE INSPECTIONS SHALL BE PERFORMED BY A SPECIAL INSPECTOR. SEE STATEMENT OF SPECIAL INSPECTION FOR COMPLETE REQUIREMENTS.

ITEM	DESCRIPTION
INSPECTION OF REINFORCING STEEL, INCLUDING PLACEMENT	FREQUENCY: PERIODIC REFERENCE: IBC 2006 TABLE 1704.4
INSPECTION/OBSERVING SPECIMEN SAMPLING OF FRESH CONCRETE FOR TESTING	FREQUENCY: CONTINUOUS REFERENCE: IBC 2006 TABLE 1704.4
CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	FREQUENCY: CONTINUOUS REFERENCE: IBC 2006 TABLE 1704.4
VERIFYING USE OF REQUIRED DESIGN MIX	FREQUENCY: PERIODIC REFERENCE: IBC 2006 TABLE 1704.4
INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS.	FREQUENCY: PERIODIC REFERENCE: IBC 2006 TABLE 1704.4
INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	FREQUENCY: PERIODIC REFERENCE: IBC 2006 SECTION 1704.4
MECHANICAL ANCHORS INSTALLED IN CONCRETE FOR PROPER APPLICATION TECHNIQUES AS REQUIRED BY MANUFACTURER	FREQUENCY: PERIODIC REFERENCE: IBC 2006 SECTION 1704.13.3
ERECTION OF PRECAST MEMBERS	FREQUENCY: PERIODIC REFERENCE: IBC 2006 SECTION 1704.4
MATERIAL VERIFICATION OF HIGH STRENGTH BOLTS, NUTS AND WASHERS; MANUFACTURER'S CERTIFICATE OF COMPLIANCE AND IDENTIFICATION MARKINGS CONFORMING TO ASTM STANDARDS SPECIFIED IN APPROVED CONSTRUCTION DOCUMENTS	FREQUENCY: PERIODIC REFERENCE: IBC 2006 TABLE 1704.3
INSPECTION OF HIGH-STRENGTH BOLTING BEARING-TYPE CONNECTIONS (JOINTS DESIGNATED AS SNUG TIGHT)	FREQUENCY: PERIODIC REFERENCE: IBC 2006 TABLE 1704.3
INSPECTION OF HIGH-STRENGTH BOLTING SLIP CRITICAL CONNECTIONS	FREQUENCY: PERIODIC REFERENCE: IBC 2006 TABLE 1704.3
INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS: BRACING AND STIFFENING, MEMBER LOCATIONS, APPLICATION OF JOINT DETAILS AT EACH CONNECTION	FREQUENCY: PERIODIC REFERENCE: IBC 2006 TABLE 1704.3
VERIFYING MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY AND THAT EXCAVATIONS ARE EXTENDED TO PROPER DEPTH	FREQUENCY: PERIODIC REFERENCE: IBC 2006 TABLE 1704.7
PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS	FREQUENCY: PERIODIC REFERENCE: IBC 2006 TABLE 1704.7
VERIFYING USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL	FREQUENCY: CONTINUOUS REFERENCE: IBC 2006 TABLE 1704.7
COMPLETE AND PARTIAL PENETRATION GROOVE WELDS	FREQUENCY: CONTINUOUS REFERENCE: IBC 2006 TABLE 1704.3 AWS D1.1, AISC 341
MULTIPASS FILLET WELDS	FREQUENCY: CONTINUOUS REFERENCE: IBC 2006 TABLE 1704.3 AWS D1.1, AISC 341
SINGLE PASS FILLET WELDS > 5/16"	FREQUENCY: CONTINUOUS REFERENCE: IBC 2006 TABLE 1704.3 AWS D1.1, AISC 341
SINGLE PASS FILLET WELDS < 5/16"	FREQUENCY: PERIODIC REFERENCE: IBC 2006 TABLE 1704.3 AWS D1.1
FLOOR AND ROOF DECK WELDS	FREQUENCY: PERIODIC REFERENCE: IBC 2006 TABLE 1704.3 AWS D1.3 AISC 341
COLD FORMED STEEL FRAMING COMPONENTS OF THE LATERAL LOAD RESISTING SYSTEM.	FREQUENCY: PERIODIC REFERENCE: IBC 2006 SECTION 1707.4

ADDITIVE ALTERNATE ONE

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DEPARTMENT OF ENGINEERING

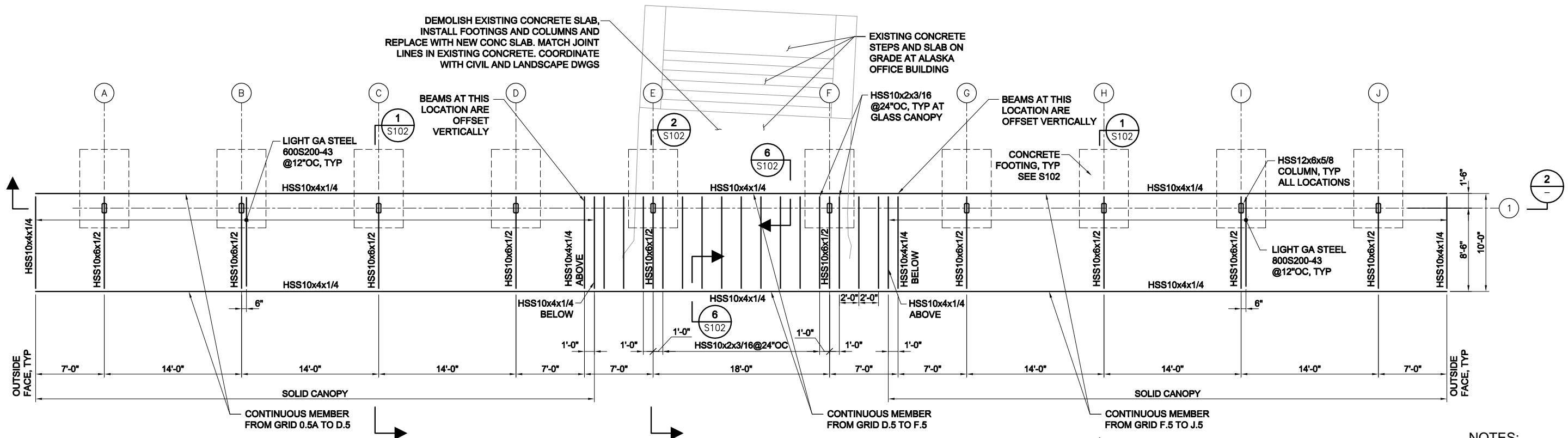
MAIN STREET IMPROVEMENTS
SECOND TO FIFTH STREET
CONTRACT NO. E12-167

GENERAL STRUCTURAL NOTES

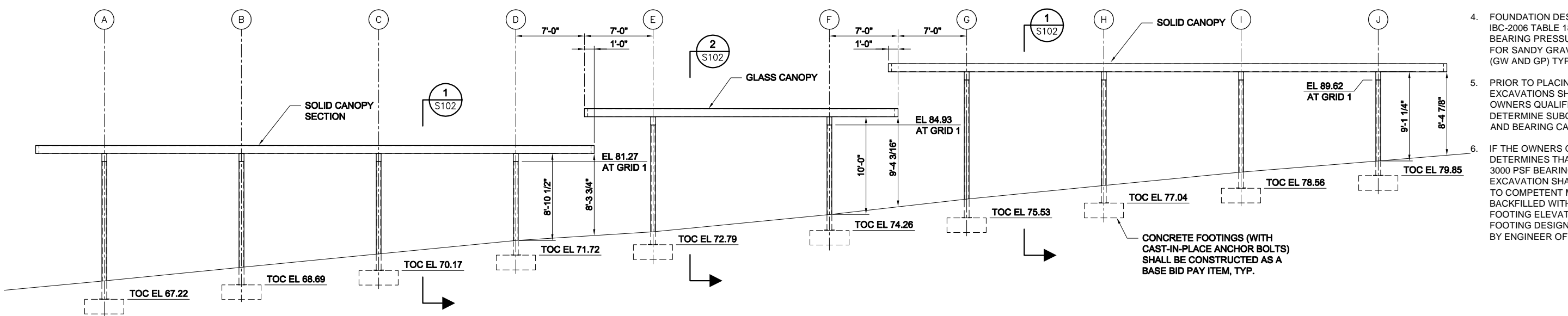
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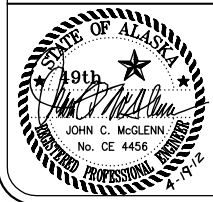
AOB CANOPIES PLAN
1
4' 0' 4' 8' 12'



AOB CANOPIES SECTION
2
4' 0' 4' 8' 12'

- NOTES:**
- CANOPIE LOCATION SHALL BE COORDINATED WITH ARCHITECTURAL DRAWINGS.
 - FINISHED GRADE PER ARCHITECTURAL AND CIVIL DRAWINGS.
 - LIGHT GAUGE STEEL MATERIAL 43 MIL (18GA) OR LIGHTER SHALL CONFORM TO ASTM A1003, GRADE ST 33H, Fy=33KSI.
 - FOUNDATION DESIGN IS BASED ON IBC-2006 TABLE 1804.2 WITH ALLOWABLE BEARING PRESSURE EQUAL TO 3000 PSF FOR SANDY GRAVEL AND/OR GRAVEL (GW AND GP) TYPE MATERIAL.
 - PRIOR TO PLACING FOOTING CONCRETE, EXCAVATIONS SHALL BE EVALUATED BY OWNERS QUALIFIED ENGINEER TO DETERMINE SUBGRADE MATERIAL TYPE AND BEARING CAPACITY.
 - IF THE OWNERS QUALIFIED ENGINEER DETERMINES THAT SUBGRADE IS NOT 3000 PSF BEARING CAPACITY, THE EXCAVATION SHALL INCREASE IN DEPTH TO COMPETENT MATERIAL AND BACKFILLED WITH CDF TO BOTTOM OF FOOTING ELEVATION, OTHERWISE FOOTING DESIGN SHALL BE ADJUSTED BY ENGINEER OF RECORD.

ADDITIVE ALTERNATE ONE



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DEPARTMENT OF ENGINEERING

**MAIN STREET IMPROVEMENTS
SECOND TO FIFTH STREET
CONTRACT NO. E12-167**

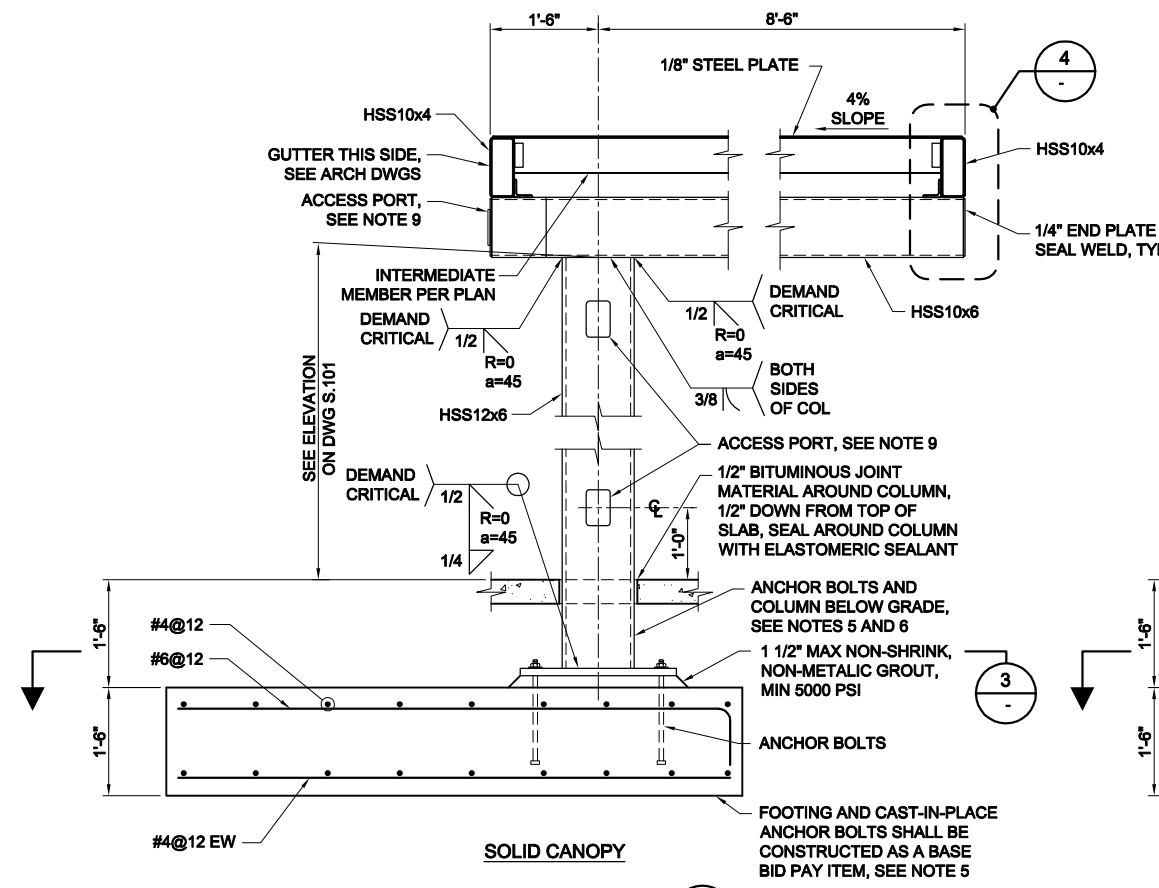
**CANOPIE
PLAN AND ELEVATION**

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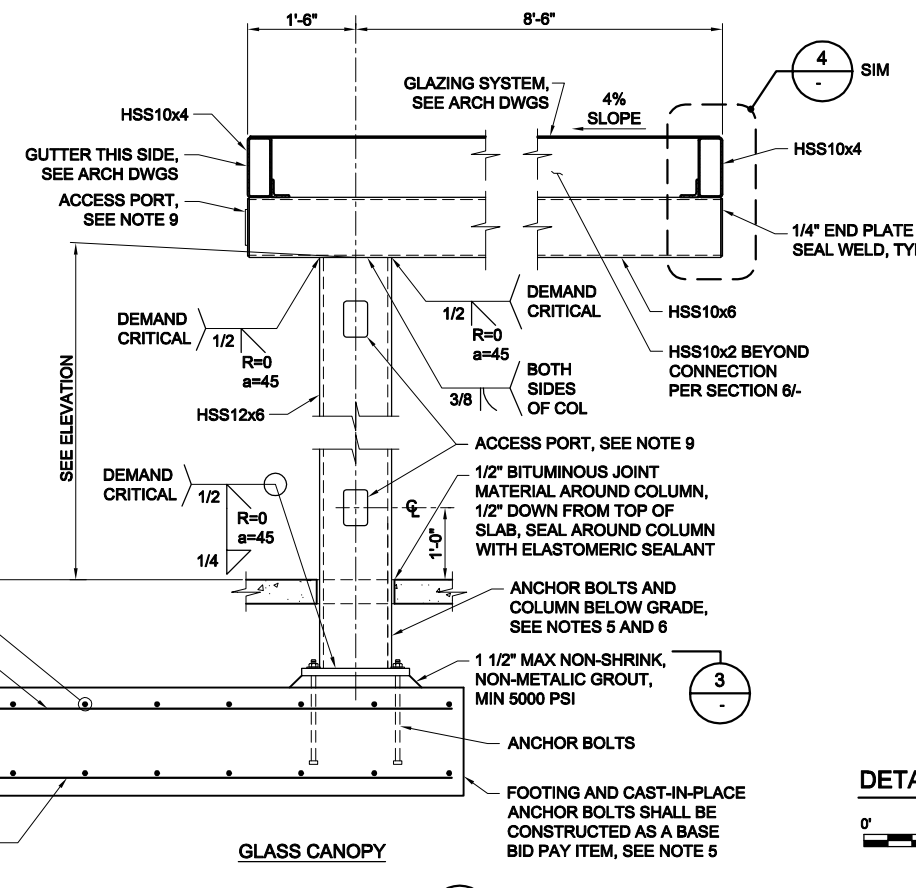
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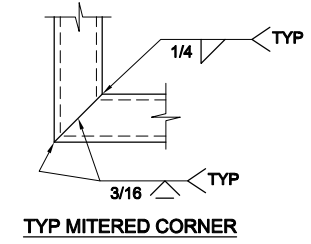
- CANOPY LOCATION SHALL BE COORDINATED WITH ARCHITECTURAL DRAWINGS.
- FINISHED GRADE PER CIVIL AND ARCHITECTURAL DRAWINGS.
- STEEL FINISH PER ARCHITECTURAL REQUIREMENTS.
- ALL EXPOSED STEEL SHALL BE CONSIDERED ARCHITECTURALLY EXPOSED STRUCTURAL STEEL FRAMING.
- AT PHASE 1 OF CONSTRUCTION, ANCHOR BOLTS SHALL BE CAST-IN-PLACE, COMPLETELY COVERED WITH MARINE GREASE AND WRAPPED WITH MINIMUM 6 MIL PLASTIC SHEETING PRIOR TO BACKFILLING. AT NEXT PHASE, ANCHOR BOLTS SHALL BE UNCOVERED AND THOROUGHLY CLEANED OF GREASE AND OTHER MATERIALS THAT WOULD PREVENT ADHESION OF BITUMINOUS PAINT.
- COLUMNS BASE PLATES AND ANCHOR BOLTS BELOW GRADE SHALL BE PAINTED WITH BITUMINOUS PAINT AFTER STRUCTURE IS ERECTED. ALLOW PAINT TO FULLY CURE PRIOR TO BACKFILLING.
- ALTERNATE CONNECTION TO USING S547 CLIP IS TO WELD 3/16x2x0'-7" PLATE TO HSS10x4 WITH 1/8" FILLET WELD AND FASTEN TO PLATE WITH 4-#10 SCREWS.
- THREADED ROD WELDED TO HSS WITH FULL PENETRATION WELD. ROD MATERIAL SHALL BE ASTM A307.
- PORTS SHALL BE CENTERED ON COLUMN, 4"x6" OPENING WITH WEATHER-TIGHT COVER PLATE, FASTENED WITH STAINLESS STEEL HARDWARE. COORDINATE LOCATION OF ELECTRICAL ACCESS PORTS AND COVER PLATE DETAILS WITH ELECTRICAL AND ARCHITECTURAL DRAWINGS.



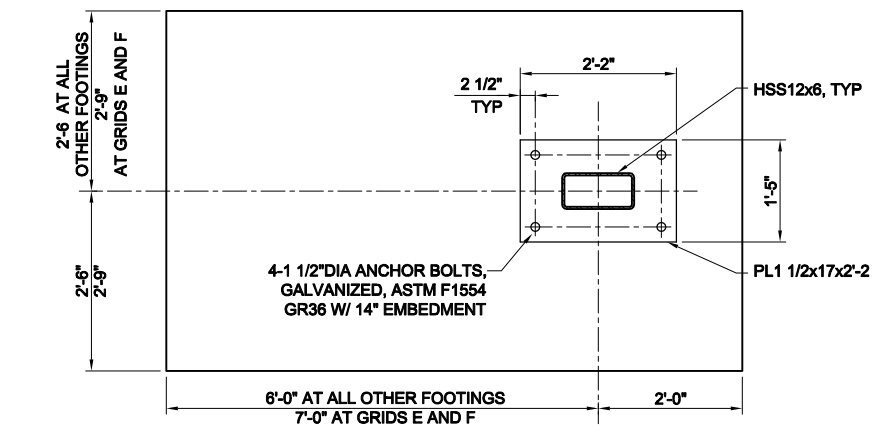
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S.101



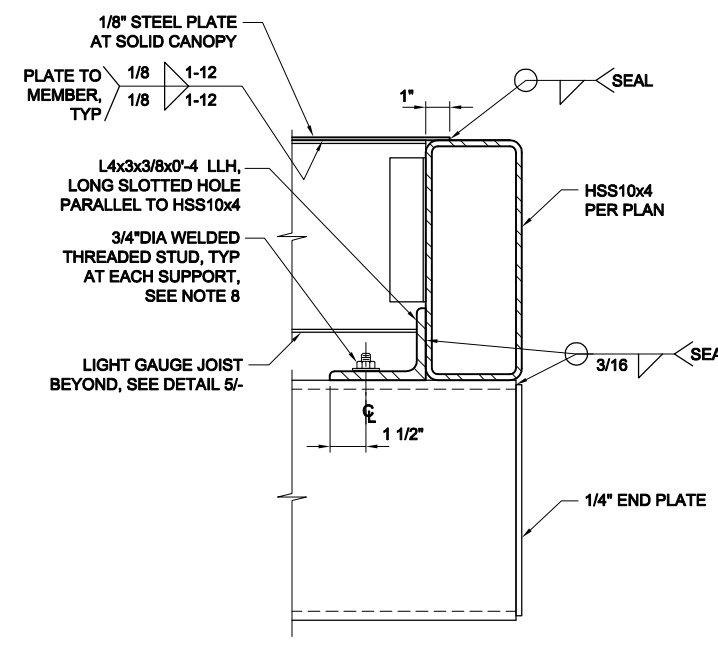
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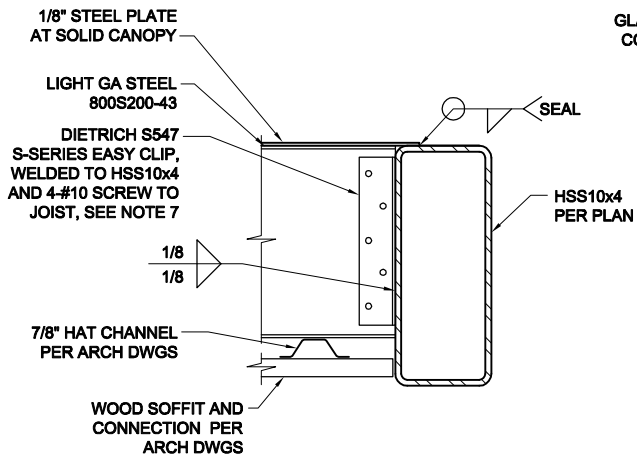
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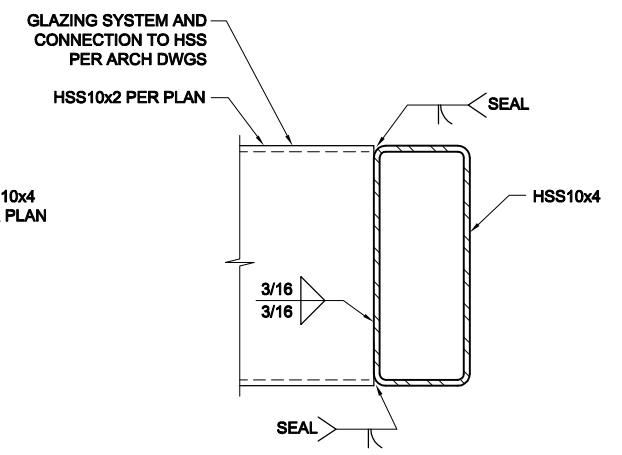
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DETAIL 4
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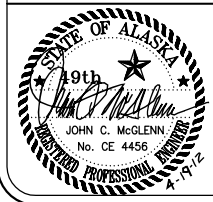


DETAIL 5
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SECTION 6
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ADDITIVE ALTERNATE ONE



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DEPARTMENT OF ENGINEERING

MAIN STREET IMPROVEMENTS
SECOND TO FIFTH STREET
CONTRACT NO. E12-167

CANOPY
SECTIONS AND DETAILS

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JOB NO: J70456 DRAWN BY: CAD DESIGNED BY: MMW CHECKED BY: DATE: MAR. 2012

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