

## APPENDIX C

### TECHNICAL MEMOS

Technical Memo #1 Landslide Mapping Accuracy and Modelling

Technical Memo #2 Landslide Designations and Boundaries – Bathe Creek and Highlands

Technical Memo #3 Mapping Overview Starr Hill Subdivision and Additional Information

Technical Memo #4 Guide to Avalanche and Landslide Hazard Designations

Technical Memo #5 Landslide Hazard Designations at Telephone Hill and Gastineau Avenue

Technical Memo #6 Severe Landslide Hazard Designations at Starr Hill and Gastineau Avenue

Technical Memo #7 Considerations for Anthropogenic Terrain at Starr Hill and Gastineau Avenue



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<b>To:</b>	Teri Camery (CBJ)	<b>Date:</b>	April 27, 2022
<b>c:</b>	Alix Pierce (CBJ)	<b>Memo No.:</b>	1
<b>From:</b>	Rita Kors-Olthof, Vladislav Roujanski, Shirley McCuaig	<b>File:</b>	704-ENG.EARC03168-01 / 704-ENG.EARC03168-02A
<b>Subject:</b>	Landslide Mapping Accuracy and Modelling Downtown Juneau Landslide and Avalanche Hazard Assessment		

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## 1.0 INTRODUCTION

This technical memo addresses some of the comments and questions that arose from Tetra Tech Canada Inc.'s (Tetra Tech) Issued-for-Review (3<sup>rd</sup> Draft) Report, Downtown Juneau Landslide and Avalanche Assessment, dated May 28, 2021, and the Landslide and Avalanche Hazard Public Meeting that took place on July 21, 2021.

The City and Borough of Juneau (CBJ) has requested a response for each of three key points, as described in CBJ's email dated July 27, 2021. This memo responds to the commentary from a local representative of the U.S. Forest Service and a mapping consultant/software/data vendor, about landslide mapping accuracy and lack of modelling.

## 2.0 LANDSLIDE MAPPING ACCURACY AND LACK OF MODELLING

Two sets of comments were received from these commenters:

- Comments dated July 21, 2021, received during the question-and-answer session of the Neighborhood Meeting (copied below).
- Comments dated August 8, 2021, received via email (summarized below).

**Question/Comment #1:** Quinn Tracy's maps and Tetra Tech's summary is clear, but the accuracy of the maps is a serious problem. Specific to the landslide hazard mapping portion of the study, there was no indication of any modern landslide modelling techniques. The references cited are over 30 years of age. Clearly efforts were focused on simply using a combination of old landslide maps and new LiDAR. Modern landslide evaluations include statistical models (calling this a statistical effort is inaccurate) and physically based models. Many models are used in the Pacific northwest and Alaska and could have been used in this study. Technically sound scientific examination of landslides, including debris slides and debris flows, would include analysis of hydrologic contributing area and evaluation of the sediment volumes in initiation and runout zones. An understanding of these parameters would aid in the understanding of landslide runout. My question to CBJ: Will you add modern landslide modelling to serve the community of Juneau?

**Question/Comment #2:** Tetra Tech's analysis provides "low, moderate, high, severe" landslide hazard zones without any quantitative description of what those hazards mean. To make rational and defensible zoning decisions requires consideration of the costs and the benefits of those decisions: comparison of the costs of precluding housing versus the probability of preventing property damage and loss of life. Why was a useful quantitative analysis conducted for snow avalanches while such an analysis was not conducted for landslides? Quantification of landslide occurrence includes utilization of the physical parameters controlling initiation and runout of landslides, establishing

probabilities of landslide initiation and runout, and an analysis of the costs entailed when an event occurs, in both dollar amount and the costs in human lives. Further, a system is needed to better inform the public of high-risk precipitation events associated with enhanced landslide activity, i.e., an early warning system for landslides.

**Response:** The first comment reflects four primary concerns, including mapping methodology and accuracy of the mapping, the age of the references, the perception that the mapping simply used a combination of old landslide maps and new LiDAR, and the lack of landslide modelling techniques. These were briefly addressed during CBJ's recorded Question-and-Answer (Q&A) session following Tetra Tech's presentation for the Landslide and Avalanche Hazard Public Meeting on July 21, 2021. The second comment reflects the desire for a quantitative analysis, a risk analysis, and an early warning system. These concerns are addressed as follows:

1. Mapping methodology and accuracy of the maps:

- a. As described in Tetra Tech's report, the mapping was completed in PurVIEW, an add-on to ArcGIS that allows the mapper to view three-dimensional (3D) air photo images on the computer screen in spatially-accurate locations. Mapping can then be completed for various air photo years with a high level of confidence in the location of the various features. For example, surficial geology was mapped at a scale of 1:2,000 to 1:4,000. This scale is a significant improvement over the scales that were available to previous mappers. For example, Swanston (1972) and Miller (1972 and 1975) would presumably have had access to air photos from 1948 at 1:40,000 scale, and from 1962 at 1:21,600 scale. Mears et al. (1992) would also have had access to some higher-resolution air photos: 1977 at 1:6,000 scale, and 1988 at 1:4,800, and appear to have used the 1977 air photos in two of their figures. However, none of these references identify the air photos used, and most do not acknowledge the use of air photos. Any images listed in Tetra Tech's Table 1.1 after 1988 would not have been available to either Swanston or Mears et al.
- b. Digital air photos were acquired from CBJ, Quantum Spatial, Inc. (QSI), the U.S. Department of Agriculture (USDA), and the U.S. Geological Survey (USGS). The air photos were georeferenced and aerially triangulated for viewing in PurVIEW. Hardcopy air photos were first scanned at high resolution (12 µm) for this purpose, and then georeferenced in 3D. Satellite and LiDAR images of the Study Area were supplied by CBJ. No mention is made in Swanston (1972) or Mears et al. (1992) of aerial imagery being ortho-rectified for use in mapping, which would have been necessary for them to reliably identify and control the locations of observed features. For example, stereo-pair images that are not ortho-rectified can have significant distortions in the images, including "compressed" or "elongated" terrain when hillslopes are viewed from different angles. This results in images that can be difficult to compare even within the same air photo year, let alone with imagery from different years. Therefore, because the old mapping was not based on ortho-rectified imagery, it was not possible for Tetra Tech to reliably overlay "old" and "new" mapping. In contrast, Tetra Tech's mapping was based on georeferenced images, allowing very accurate overlays of the different years of imagery.
- c. Surficial geology was mapped using the 1948 air photos to provide a baseline for the maps that extends as far back in time as the air photo coverage of the Study Area allows. Given the limited capabilities of photographic equipment in 1948, the 1962 air photos were used to check the base historical mapping and the surficial geology mapping. Then, the 1962 and later air photos and satellite images were used to determine slide activity visible on the dates of those images, using lack of vegetation as a proxy for slide activity.
- d. The LiDAR bare-earth hillshade model images were primarily used to refine and show the locations of such major terrain features as gullies and debris flow fans. Due to the high resolution of the LiDAR data, it was possible to map a large number of gullies. Gully erosion, as a hazardous geomorphic process, was given close attention in this landslide hazard assessment study because gully erosion plays a significant role in mass movement on the slopes, with some of the gullies being conduits for conveying debris flows, debris slides, and wet avalanches.

- e. Historical records and incident reports, as well as contemporary photographs and news reports, were used to supplement the mapping in specific localities. However, the main components of the mapping are based on the historical air photo record review, the LiDAR images review, and the fieldwork completed by Tetra Tech.
  - f. Preliminary field maps were prepared for use during the site reconnaissance visit and were updated in accordance with the observations made in the field.
  - g. The site reconnaissance included the following tasks:
    - i. A helicopter fly-over of the Study Area was conducted to provide a wider perspective of suspected areas of slope instability, to target specific areas for ground-truthing, and to provide access to otherwise inaccessible or difficult-to-access areas.
    - ii. A foot-traverse inspection of a large portion of the Study Area was done for field mapping of landslide areas and ground-truthing of geomorphic features/hazards (e.g., landslides), key terrain features, and vegetation damage (slope instability-related) identified from air photo and LiDAR data analysis.
    - iii. Measurements, photographs, and Global Navigation Satellite System (GNSS such as GPS/GLONASS) data were collected for landslide initiation and runout zones to help define hazard types and mechanisms.
    - iv. Additional emphasis was placed on field observations in residential areas, resulting in a much greater density of field observations and time spent in residential subdivisions, e.g., the Behrends, White, and Starr Hill Subdivisions.
  - h. Several landslide events that occurred subsequent to the completion of Tetra Tech's mapping served to confirm the accuracy of the mapping.
2. The references cited were over 30 years of age:
- a. Numerous references cited are less than 30 years of age.
  - b. Age alone is not considered a valid reason to reject the use of references that provide valuable information for the project. For example, some of the older references provided very useful historical context that would have otherwise required considerably more research to acquire.
  - c. All references, including those that were over 30 years old, were evaluated for quality in accordance with the technology that was available at the time, and used or referenced (or not) as appropriate for the goals of the current mapping project.
3. The perception that the mapping simply used a combination of old landslide maps and new LiDAR:
- a. It is clear from the description of the actual mapping methods presented in the report, and from the summary provided above, that this perception is incorrect.
  - b. This is not to say that the old mapping was ignored in the production of the new mapping. The old mapping, from several sources, was reviewed to confirm that landslides presented in the older work were either represented in the new mapping (and appropriately updated to the present day, if/as required); *OR* that specific features had been reviewed and, on the basis of findings using higher-resolution technology, considered not applicable.
4. Lack of landslide modelling techniques:
- a. Landslide modelling was not in the project scope.

- b. Geotechnical drilling was not in the project scope.
- c. For a landslide model to provide an estimate of landslide runout more convincing than that already provided by the direct evidence seen on the ground or from air photos would require significantly more effort than was feasible with the available project funding. This judgment is confirmed by the comparison of Tetra Tech's mapping with a set of slope stability models prepared by others for a local Juneau watershed.
- d. If Tetra Tech were to carry out landslide modelling on selected debris slides, debris flows, rockfalls, or rockslides (for example), the scope would require not just modelling, but the collection of additional supporting field data, including, but not limited to:
  - i. Detailed engineering geology mapping for rockfalls or rockslides, including identification of structural domains, faults, discontinuity sets/orientations, rock mass quality;
  - ii. Collection of detailed topographic data, preferably including a topographic survey; information on surface conditions including vegetation, surface drainage, signs of ponding or erosion, tension cracks, observations of ground deformations etc.; field identification of initiation and runout zones; characteristics and performance of adjacent or nearby slopes; identification of landslide terrain that contributes to debris flows; noting possible changes since the previous inspection;
  - iii. Detailed characteristics of suspected or known debris flow gullies, such as upslope gradients and/or terrain stability class; stepped gully configuration (i.e., sediment stored in debris wedges); debris flow levees, avulsions; fan destabilization potential as indicated by number of channels, degree of incision; water transport potential as indicated by channel width, size/presence of woody debris, maximum sediment particle size; consideration of headwall/sidewall failure potentials based on slope gradients and surficial materials, gully geometry potential for debris flows based on sidewall slope lengths and channel gradients;
  - iv. Geotechnical drilling and/or testpitting, potentially with several testholes at each site location. Depending on location, achieving access could require tracked drills or heli-portable drills;
  - v. Collection of soil/rock samples from boreholes or test pits. Successful sampling will depend on the anticipated materials to be sampled and on the choice of sampling method, e.g., drill type;
  - vi. Installation and long-term monitoring of instrumentation such as slope inclinometers, piezometers, and remote access data acquisition systems;
  - vii. Laboratory index testing to classify and determine engineering properties of soils, and strength testing on selected samples (soil or rock);
  - viii. Analysis and modelling, potentially including (depending on the type of landslide):
    - 1. Visual slope retrogression analysis based on air photos and current site observations;
    - 2. Semi-quantitative slope analysis, beginning with back-analysis to determine the slope parameters (several models available for evaluation); and/or
    - 3. Debris flow analysis (several models available for evaluation).
  - ix. A detailed geotechnical investigation, instrumentation monitoring and analysis/modelling program could require an additional budget ranging from \$250,000 to \$1,000,000 per site to be investigated, depending on the complexity of the landslide and access, the type of drill required and where it is mobilized from, and the instrumentation to be installed. Each site also requires long-term monitoring and data analysis, at an additional annual cost that could reach \$125,000 to \$500,000. Tetra Tech notes that mobilizing a suitable drill from Whitehorse, Anchorage, or further away, would entail

significant costs. For example, for two Alaska Department of Transportation projects with challenging access conditions (the Juneau Access Road and a new section of the Sterling Highway), a geotechnical drilling contractor from Washington State conducted the exploration work. It is anticipated that to further investigate and analyze even a few sites would rapidly result in a budget exceeding several million dollars; and

- x. It is noted that Tetra Tech conducted a semi-quantitative analysis specifically to compare various geotechnical parameters and associated landslide prevalence against the hazard designation categories. This was not intended to be a detailed statistical analysis, for example, such as could have been prepared based on the results of a much more extensive field investigation throughout the map area, including a geotechnical drilling program in selected locations. Accounting for the high resolution with which the surficial geology and landslide mapping was accomplished, and the proven accuracy of that mapping as seen from later landslide events that confirmed the mapping, the semi-quantitative analysis is considered to have been a value-added contribution to the mapping process.

5. Lack of quantitative analysis:

- a. Determination of “the physical parameters controlling initiation and runout of landslides” requires the acquisition of additional site-specific information, which was not in the project scope.
- b. See Item 4 above.

6. Risk analysis:

- a. A risk analysis includes not only an assessment of hazards; but also consequences, e.g., costs related to property damage, injury, or loss of life; and the resulting risk.
- b. Determination of the probabilities of landslide initiation and runout is a task that would be greatly facilitated with the acquisition of more site-specific information, which was not in the project scope (see Items 4 and 5 above).
- c. This task would also entail a magnitude-frequency analysis, which was not in the project scope.
- d. The determination of consequences and risk were not in the project scope.

7. Early warning system:

- a. The development of an early warning system would require a detailed analysis of climate and climate change, which was not in the project scope.
- b. Development of the early warning system itself was not in the project scope.

## 3.0 LIMITATIONS OF REPORT

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## 4.0 CLOSURE

We trust this technical memo meets your present requirements. If you have any questions or comments, please contact the undersigned.

Respectfully submitted,  
Tetra Tech Canada Inc.



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## GEOTECHNICAL

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The Client, and any Authorized Party, acknowledges that the Professional Document is based on limited data and that the conclusions, opinions, and recommendations contained in the Professional Document are the result of the application of professional judgment to such limited data.

The Professional Document is not applicable to any other sites, nor should it be relied upon for types of development other than those to which it refers. Any variation from the site conditions present, or variation in assumed conditions which might form the basis of design or recommendations as outlined in this report, at or on the development proposed as of the date of the Professional Document requires a supplementary investigation and assessment.

TETRA TECH is neither qualified to, nor is it making, any recommendations with respect to the purchase, sale, investment or development of the property, the decisions on which are the sole responsibility of the Client.



## 1.7 ENVIRONMENTAL AND REGULATORY ISSUES

Unless stipulated in the report, TETRA TECH has not been retained to investigate, address or consider and has not investigated, addressed or considered any environmental or regulatory issues associated with development on the subject site.

## 1.8 NATURE AND EXACTNESS OF SOIL AND ROCK DESCRIPTIONS

Classification and identification of soils and rocks are based upon commonly accepted systems and methods employed in professional geotechnical practice. This report contains descriptions of the systems and methods used. Where deviations from the system or method prevail, they are specifically mentioned.

Classification and identification of geological units are judgmental in nature as to both type and condition. TETRA TECH does not warrant conditions represented herein as exact, but infers accuracy only to the extent that is common in practice.

Where subsurface conditions encountered during development are different from those described in this report, qualified geotechnical personnel should revisit the site and review recommendations in light of the actual conditions encountered.

## 1.9 LOGS OF TESTHOLES

The testhole logs are a compilation of conditions and classification of soils and rocks as obtained from field observations and laboratory testing of selected samples. Soil and rock zones have been interpreted. Change from one geological zone to the other, indicated on the logs as a distinct line, can be, in fact, transitional. The extent of transition is interpretive. Any circumstance which requires precise definition of soil or rock zone transition elevations may require further investigation and review.

## 1.10 STRATIGRAPHIC AND GEOLOGICAL INFORMATION

The stratigraphic and geological information indicated on drawings contained in this report are inferred from logs of test holes and/or soil/rock exposures. Stratigraphy is known only at the locations of the test hole or exposure. Actual geology and stratigraphy between test holes and/or exposures may vary from that shown on these drawings. Natural variations in geological conditions are inherent and are a function of the historic environment. TETRA TECH does not represent the conditions illustrated as exact but recognizes that variations will exist. Where knowledge of more precise locations of geological units is necessary, additional investigation and review may be necessary.

## 1.11 PROTECTION OF EXPOSED GROUND

Excavation and construction operations expose geological materials to climatic elements (freeze/thaw, wet/dry) and/or mechanical disturbance which can cause severe deterioration. Unless otherwise specifically indicated in this report, the walls and floors of excavations must be protected from the elements, particularly moisture, desiccation, frost action and construction traffic.

## 1.12 SUPPORT OF ADJACENT GROUND AND STRUCTURES

Unless otherwise specifically advised, support of ground and structures adjacent to the anticipated construction and preservation of adjacent ground and structures from the adverse impact of construction activity is required.

## 1.13 INFLUENCE OF CONSTRUCTION ACTIVITY

There is a direct correlation between construction activity and structural performance of adjacent buildings and other installations. The influence of all anticipated construction activities should be considered by the contractor, owner, architect and prime engineer in consultation with a geotechnical engineer when the final design and construction techniques are known.

## 1.14 OBSERVATIONS DURING CONSTRUCTION

Because of the nature of geological deposits, the judgmental nature of geotechnical engineering, as well as the potential of adverse circumstances arising from construction activity, observations during site preparation, excavation and construction should be carried out by a geotechnical engineer. These observations may then serve as the basis for confirmation and/or alteration of geotechnical recommendations or design guidelines presented herein.

## 1.15 DRAINAGE SYSTEMS

Where temporary or permanent drainage systems are installed within or around a structure, the systems which will be installed must protect the structure from loss of ground due to internal erosion and must be designed so as to assure continued performance of the drains. Specific design detail of such systems should be developed or reviewed by the geotechnical engineer. Unless otherwise specified, it is a condition of this report that effective temporary and permanent drainage systems are required and that they must be considered in relation to project purpose and function.

## 1.16 BEARING CAPACITY

Design bearing capacities, loads and allowable stresses quoted in this report relate to a specific soil or rock type and condition. Construction activity and environmental circumstances can materially change the condition of soil or rock. The elevation at which a soil or rock type occurs is variable. It is a requirement of this report that structural elements be founded in and/or upon geological materials of the type and in the condition assumed. Sufficient observations should be made by qualified geotechnical personnel during construction to assure that the soil and/or rock conditions assumed in this report in fact exist at the site.

## 1.17 SAMPLES

TETRA TECH will retain all soil and rock samples for 30 days after this report is issued. Further storage or transfer of samples can be made at the Client's expense upon written request, otherwise samples will be discarded.



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<b>To:</b>	Teri Camery (CBJ)	<b>Date:</b>	April 27, 2022
<b>c:</b>	Alix Pierce (CBJ)	<b>Memo No.:</b>	2
<b>From:</b>	Rita Kors-Olthof, Vladislav Roujanski, Shirley McCuaig	<b>File:</b>	704-ENG.EARC03168-01 / 704-ENG.EARC03168-02A
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## 1.0 INTRODUCTION

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The City and Borough of Juneau (CBJ) has requested a response for each of three key points, as described in CBJ's email dated July 27, 2021. This memo responds to commentary from a local avalanche expert, as well as from other residents with questions about the Bathe Creek and/or Highlands mapping areas.

## 2.0 LANDSLIDE DESIGNATIONS AND BOUNDARIES

Some detailed commentary about the mapping was provided to CBJ in a letter dated July 26, 2021. The writer deemed the avalanche mapping generally accurate and well-done. The remaining commentary was mostly concerned with landslides, offering some general critique for the overall landslide mapping, as well as some specific observations in the Bathe Creek and Highlands areas. Questions and concerns have been documented below in a question-and-answer format beginning with general mapping questions, followed by Bathe Creek/Highlands-specific questions from several people. In cases where questions were similar or related, these have been combined for the response.

### Overall Landslide Mapping:

1. **Question/Comment:** Incorrect classifications based on recorded return intervals:
  - a. Areas shown as *Severe* that have recorded frequencies in the *High* range; and
  - b. Areas that have become inactive as drainages have changed, suggesting that they should be designated as *Moderate* to *High*, instead of *Severe*, if the stated standard of mapping for current conditions is followed.

**Response:** As noted in Section 1.2.4.2.5 of the main report, the analysis of magnitude/frequency was *not* part of this high-level study. There appears to be reasonably good correspondence between the levels of historical activity identified and typical event probabilities, particularly in many of the highly-active landslide features. For that reason, Tables 1.3, 1.4, and B.1 in the main report include some very high-level estimates that could be helpful for visualizing the differences between the hazard designations. These estimates have now been revised to correspond more closely to the format of the estimates used in the avalanche study. However, it should be noted that these estimates are based solely on the level of activity observed in slope movement features and gully erosion features identified from the historical air photo record analysis and the LiDAR data analysis, as well as field observations and, where available, incident reports. A magnitude-frequency analysis will be required to produce more reliable estimates for event probabilities than is currently possible. See “A Guide to Avalanche and Landslide Hazard Designations” for additional information and examples (Technical Memo #4 in Appendix C of the main report; Tetra Tech 2022d).

It should also be noted that the frequency or return period of an event (or the mapping proxy of visual evidence of repeated slide activity) does not mean that an event of a specified size or severity will return every X number of years. For example, a debris flow of a certain size typically depends on two events coinciding: a storm event large enough to mobilize debris in a gully, and enough debris accumulated in the gully from previous events to mobilize the debris. Furthermore, a 1 in 30-year rainstorm event (for example) could happen at *any* time in a 30-year period. It could happen this year, and it could also happen again next year. (Though if it keeps happening, the meteorologists might eventually decide that the new normal for that size of storm is a 1 in 10-year event.)

In general, the hazard designation mapping is not intended to indicate event probability (whether an event is likely to occur within a specified number of years), but rather whether a hazard is expected at some point in the future, generally based on evidence that it has already done so in the past. These maps provide the locations of hazardous areas, which is important information for planning purposes. More detail, such as determining the potential frequency and magnitude of the events that could occur in those areas is a logical next step; however, it is not without significant cost.

Additionally, an area is given a hazard designation of *Severe* if:

- A cone or fan of colluvium is present at the base of a slope, no matter how old it is, because the hazard still exists (Howes and Kenk 1997); and/or
- Evidence of slope instability (exhibited on air photos as a lack of vegetation in a formerly vegetated area with an obvious downslope movement component; incident reports; and/or field observations) is identified within the same feature in more than one air photo/LiDAR year and/or field investigation year.

Note that numerous gullies show evidence of slope instabilities in several years (sometimes every year) of imagery, incident report data, or field observation data that was reviewed.

In one area, located southeast of Snowslide Creek near the top of slope, visual evidence of bedrock movement seen at ground surface during the fieldwork indicated the possibility of an impending deep-seated bedrock slide that could reach Gastineau Channel. Because it has not yet failed, a level of activity could not be determined for that site but, due to the very large size of the feature and the notable consequence of its failure, it was given a *Severe* hazard designation.

For changed drainages, or map areas that appear not to have had an obvious slope instability for decades, e.g., 60 years, this does *not* mean that the area is now “inactive.” Also, there may be other factors that account for the feature being designated as a higher level of hazard, including the type of feature and the amount of historical instability on the slopes above it.

Debris cones (steeper conical features) and fans (more gently sloping fan-shaped features) develop by the accumulation of unconsolidated surficial material (debris) that is transported and deposited in ever-shifting flow channels that migrate from one side of the cone/fan to the other. This is how the fan or cone shape forms. A debris flow entering at the top (“apex”) of a cone is capable of flowing *anywhere* on that cone, even if it has flowed for many years in the same incised channel. A new debris flow provides its own, sometimes very viscous clayey/silty soil matrix, which can incorporate or entrain boulders and large woody debris, and it can block its own direction of movement with that material. Unless the flow can be controlled and diverted from the very top of the feature (usually an expensive proposition), what happens below the apex is not entirely predictable. The behaviour of debris flows that are not well-incised is even less predictable. The smallest disruption in the ground surface, like a fallen tree or a bit of debris blocking the usual flow direction, can result in an abrupt change of flow direction, or a splitting of the flow into several channels.

- Question/Comment:** Placement of very different hazard designations together, e.g., *Severe* next to *Low*, rather than the designations progressing downslope from *Severe-High-Moderate-Low*, that the writer would have expected with the higher frequency of smaller, shorter-running slides.

**Response:** Landslide hazards are generally not downgraded in a downslope direction. If there is a history of slide activity, or if it is a runout zone (i.e., a deposition zone), an area is considered to pose a *Severe* hazard. Note that not all landslide events begin at the top of the mapped feature, nor do they necessarily extend to the bottom of the feature, which can be seen clearly in the historical air photo record and LiDAR data analysis, in both the slope movement features (Figures 1.4a to 1.4c) and the gully erosion features (Figures 1.5a to 1.5c). Landslide modelling could be used to refine the runout zones and potentially downgrade the designated hazards in some locations. However, as noted in Technical Memo #1 (in Appendix C of the main report; Tetra Tech 2022a), landslide modelling was not part of the scope of this project.

- Question/Comment:** Mapping polygons appear oddly-shaped, not corresponding to the lobate flow features or runouts expected in areas without strong topographic controls. Areas of criticism include “odd little pointy bits,” and abrupt changes in direction.

**Response:** In several instances, the apparent odd shapes of some of the terrain units result from the shapes of the adjacent terrain units. One example is the Bathe Creek drainage above Irwin Street and Gold Creek, where the shape of the east edge of the colluvial cone/fan is affected by an area of bedrock (shown in purple and marked Rr/Cv on Figure 1.3b). Immediately upslope of the bedrock area is another area of colluvium marked Ca|dr that apparently encroaches into the Bathe Creek drainage. So, even though the Bathe Creek fan/cone looks a little odd on the east side, the adjacent landforms are the cause of this peculiar boundary (see excerpts of the figures in Item 6 below).

- Question/Comment:** Deglaciation was relatively recent in our region, and the retreat of the ice was followed by a period of enhanced mass wasting, as the glacially-oversteepened slopes came to a new equilibrium and became vegetated. Most of the present-day colluvium dates back to that period. The bulk of the fan area of concern would appear to be from then. The volume exceeds what would have likely come from Bathe Creek; it is far more likely that the bulk of the material came from the steep slopes directly above Evergreen, than from cross-slope movement from Bathe Creek. It is a more reasonable interpretation that the recent activity apparent from air photos and LIDAR is the surface veneer, showing only the most recent activity, now obscured by development.

**Response:** Tetra Tech respectfully disagrees, given the amount of activity seen in the historical landslide mapping. If a series of small debris slides or flows occurs in the same area repeatedly, there is a hazard. It may be smaller than the hazard of major slides or flows that are very old, but it is still a hazard to the slopes below. Loose material builds up on the slopes with each small event, eventually leading to a larger event that incorporates that material and adds some of its own. Whether a landslide consists of a debris slide or a debris

flow, and regardless of the source of the materials (e.g., “old” colluvium or “new” colluvium), it is a significant hazard to the slopes below and should not be downgraded.

### Questions/Comments – Bathe Creek along Irwin Street and up Highlands:

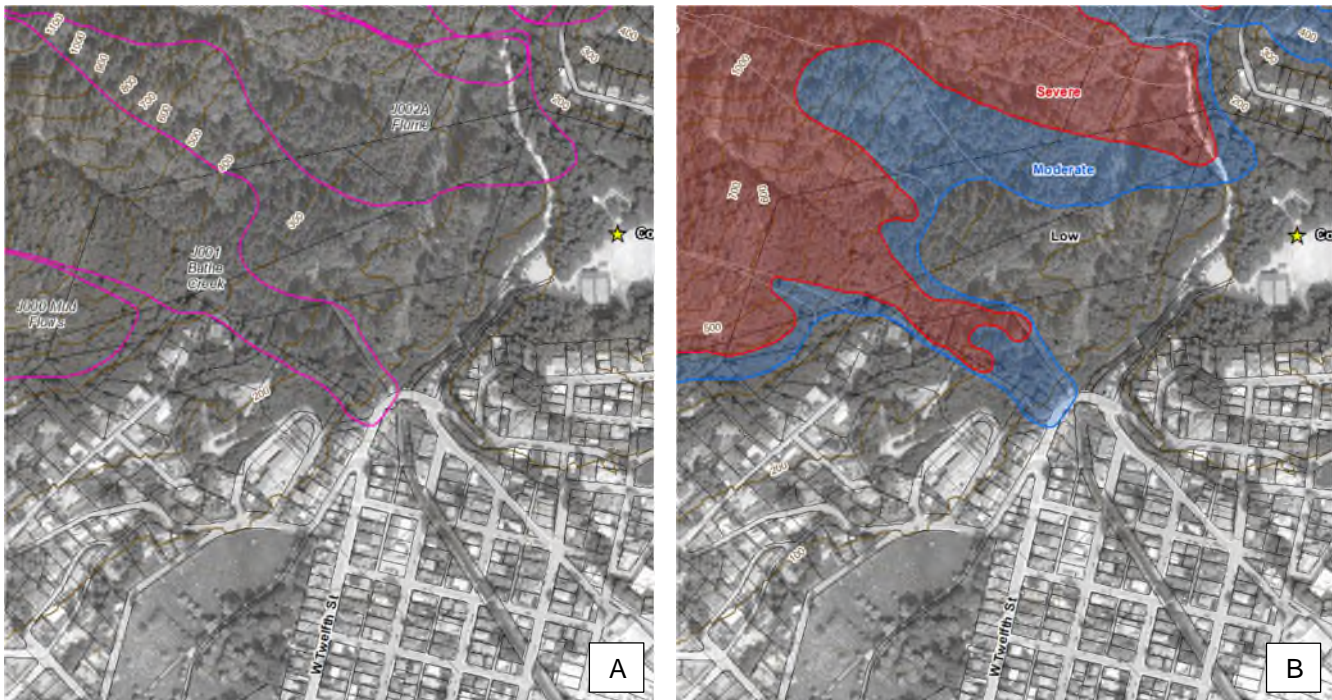
To document their local knowledge of the Bathe Creek / Irwin Street / Highlands area, the writer reported that they have lived since the early 1990’s on the upper part (northeast end) of 12<sup>th</sup> Street. Due to the lack of mapping at that time, the writer explored the slopes above this area, as well as the slopes throughout the Highlands area, during their house-hunting efforts. The following site-specific questions/comments were provided.

5. **Question/Comment:** Bathe Creek is known to produce debris flows, but the historical frequency is only in the 10- to 30-year range, not the 1- to 10-year range suggested in the report for a *Severe* designation. The writer suggests that the historical frequency is high enough to even out possible anomalies and that the return interval is the most reliable basis for mapping hazard.

**Response:** See response to Item 1 above for general remarks about frequency and historical activity. Specific to Bathe Creek, this feature is a very active debris flow gully, according to the historical air photo record and LiDAR analysis, and it is therefore rated *Severe*.

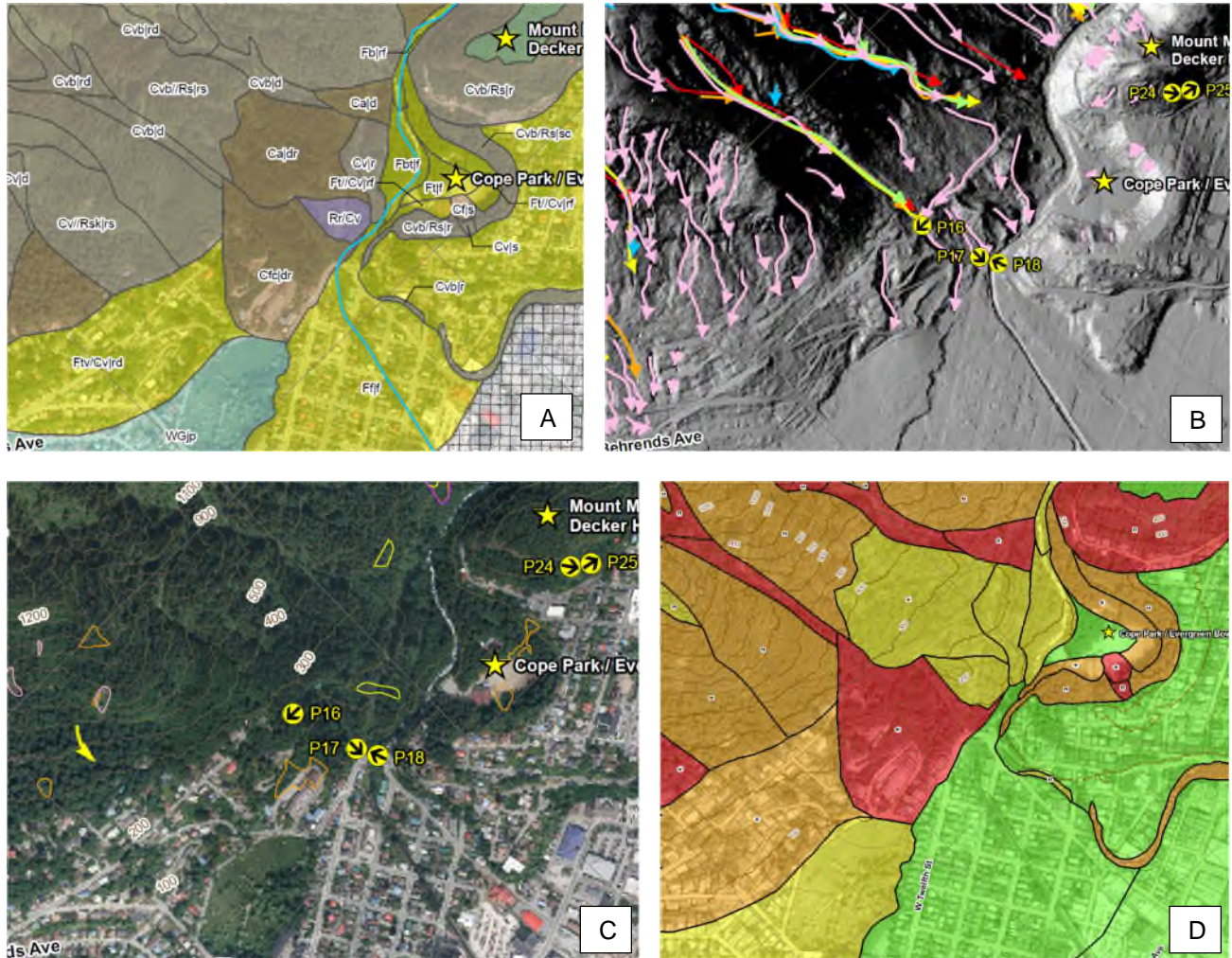
6. **Question/Comment:** The mapped boundaries [of the *Severe* hazard zone] are noted to be irregular, with curlicues and projections on the [east] side that do not resemble the mapped lobate avalanche boundaries, which may better define the landslide hazard area.

**Response:** See response to Item 3 above regarding the irregular “curlicues and “projections.” Bathe Creek is an example of where adjacent terrain units affect the shape of this terrain unit. In general, because the processes are not the same, avalanche path boundaries should not be expected to match landslide hazard boundaries. Compare the excerpts from the avalanche path and hazard mapping in Figures 2.2c and 2.3a (Figure 1 below) with the surficial geology and major gully features in Figures 1.3b and 1.5b (Figure 2 below). The boundaries of wet avalanches might sometimes approximate the boundaries of debris flows in the same terrain feature, a resemblance that would necessarily depend on the size and mobility of each event. In this case, they could be similar but not identical.



**Figure 1: Excerpts from Figure 2.2c Avalanche Path Mapping Detail (Figure 1A) and Figure 2.3a Avalanche Hazard Designation Mapping Overview (Figure 1B).**

Compare the surficial geology and the major gully features shown in excerpts from Figures 1.3b and 1.5b (Figures 2A and 2B below). In the case of Bathe Creek, the shape of the *Severe* area reflects the shape of the cone/fan and the upslope gully that is the source of the debris, as shown below in the excerpt from Figure 1.6c (Figure 2D). Finally, even though the fan itself only displays occasional activity (remobilization of old debris flow material), as shown in the excerpt from Figure 1.4b (Figure 2C), the debris flows originating from upslope govern the hazard designation, as they are what has formed the fan and what will continue to do so in the future.



**Figure 2: Excerpts from Figure 1.3b Surficial Geology (Figure 2A) and Figure 1.5b Historical Air Photo Record and LiDAR Data Analysis – Gully Erosion Features (1948-2013) (Figure 2B), and excerpts from Figure 1.4b Historical Air Photo Record – Slope Movement Features (1948-2020) (Figure 2C) and Figure 1.6c Landslide Hazard Designation Mapping (Figure 2D).**

7. **Question/Comment:** Swanson’s study seems to indicate that flow from Bathe Creek was turning sharply enough to make that south and west area part of the runout, either when he did his 1972 study, or in the recent past. But the topography at the mouth of the Bathe Creek canyon is now incised enough that it would take a major change to divert it south and west again. This mapping is supposed to be for current conditions and topography, not speculation on how it might change in the future.

**Response:** See the last paragraph in the response to Item 1, which describes how debris flow cones and fans are formed. Specific to Bathe Creek, the mouth of the creek is not the location where the most significant changes in flow direction are likely to occur. Instead, it would be the apex of the cone/fan, i.e., above Evergreen Avenue. See Item 6.

8. **Question/Comment:** Residents of the area extending from above the east end of upper Evergreen Avenue downslope through Hermit Street, Rheinhardt Street, and Irwin Street, are concerned about the *Severe* hazard designated for this area as a result of the new mapping, instead of the *Moderate* hazard that applied before in most (but not all) of the subdivision. The changes in assumptions between the 1987 adopted hazard maps and

the proposed 2021 maps are not well understood, resulting a lack of confidence in the results. A few residents thought that the new study was based on the old study. Some residents stated that they would not have purchased their property if they had known that it was in a *Severe* hazard zone. Another resident on Hermit Street owned a house whose original owner had a site-specific review done prior to construction for their proposed building site, due to the previous mapping (Swanston 1972) that had identified a hazard in the area. The current owner has commented that they might do the same.

Some residents also noted that they were not aware of a landslide event that had affected their address, nor did it seem to them that there had been a change in topography or vegetation in the area over the years. Other residents acknowledged the presence of the nearby Bathe Creek gully but felt that the topography protected the residential areas from landslides or avalanches. A few residents wondered if existing structures or trees would reduce the likelihood of impacts from landslides or avalanches.

How can homeowners in this area reconcile the old mapping with the new mapping, and better understand what the new mapping means?

**Response:** Additional areas of *Severe* and *High* hazard were added to the hazard map, because of the types of landforms and the amount of landslide activity that was seen on the air photos, on the LiDAR images, and during the fieldwork. A *High* or *Severe* hazard designation is not always well represented by what one sees happening (or not happening) on the slopes adjacent to one's property. The hazard designation may have been assigned because of what is happening on the slope well upslope of a particular property. This is true for Bathe Creek and the surrounding terrain. See the last paragraph of the response to Item 1, and also Items 4 and 7. The debris flow potential of the slope above is one part of the rationale that results in the *Severe* hazard designation for this area. The debris flow paths on the fan/cone can easily be shifted by a debris flow if something happens higher up near the fan/cone apex that changes the direction of flow. This area was reviewed in detail during the field work.

The debris initiation and runout zones appear to have been missed in the Swanston (1972) study, although the main gully was identified as being a *High* hazard path (the same as the 1987 *Severe* hazard designation) from the east end of Evergreen Avenue all the way to Gold Creek. Swanston identified superimposed deposits along the main gully that showed repeated landslide activity. He also commented on two debris flows that ran an hour apart down the gully, badly damaging a home and filling Irwin Street with debris. Swanston further noted that there were two smaller gullies that led down into the gravel quarry (now the residential area) above Martin Road. Tetra Tech notes that this residential area has all the hallmarks of a runout area, since it is downslope of the initiation and transition zones of a prominent and very active debris slide/debris flow path.

It seems possible, if not likely, that the presence of the former gravel pit obscured some of the signs of the debris flow fan at the time of Swanston's study. However, a soils study conducted prior to the development of the Westridge Condominium project, carried out by R&M Engineering in 1980 and 1981, encountered 10 feet of silty sand colluvium upslope of the project, at about 160 feet above sea level. The back of the condo project was noted to be at about 140 feet elevation. Ten feet of colluvium is not an insignificant amount of material. Beneath the colluvium was sandy gravel of glaciofluvial/deltaic origin, which would have been the type of material preferred for gravel pit operations. Notably, if the residential area is located within the former gravel pit (as it is understood to be), R&M Engineering would not have encountered the colluvium there, because it would have been stripped off the site before excavating the gravel for use. Once again, the presence of a hazard from upslope was not recognized.

Further consultation did not reveal any further reasons for caution (Swanston 1990), since the 1972 study had been intended for use as a planning-level tool, not a site-specific investigation. He also felt that the "alluvial cone deposits" beneath the property, and the bedrock-controlled gully would keep avalanches or debris flows

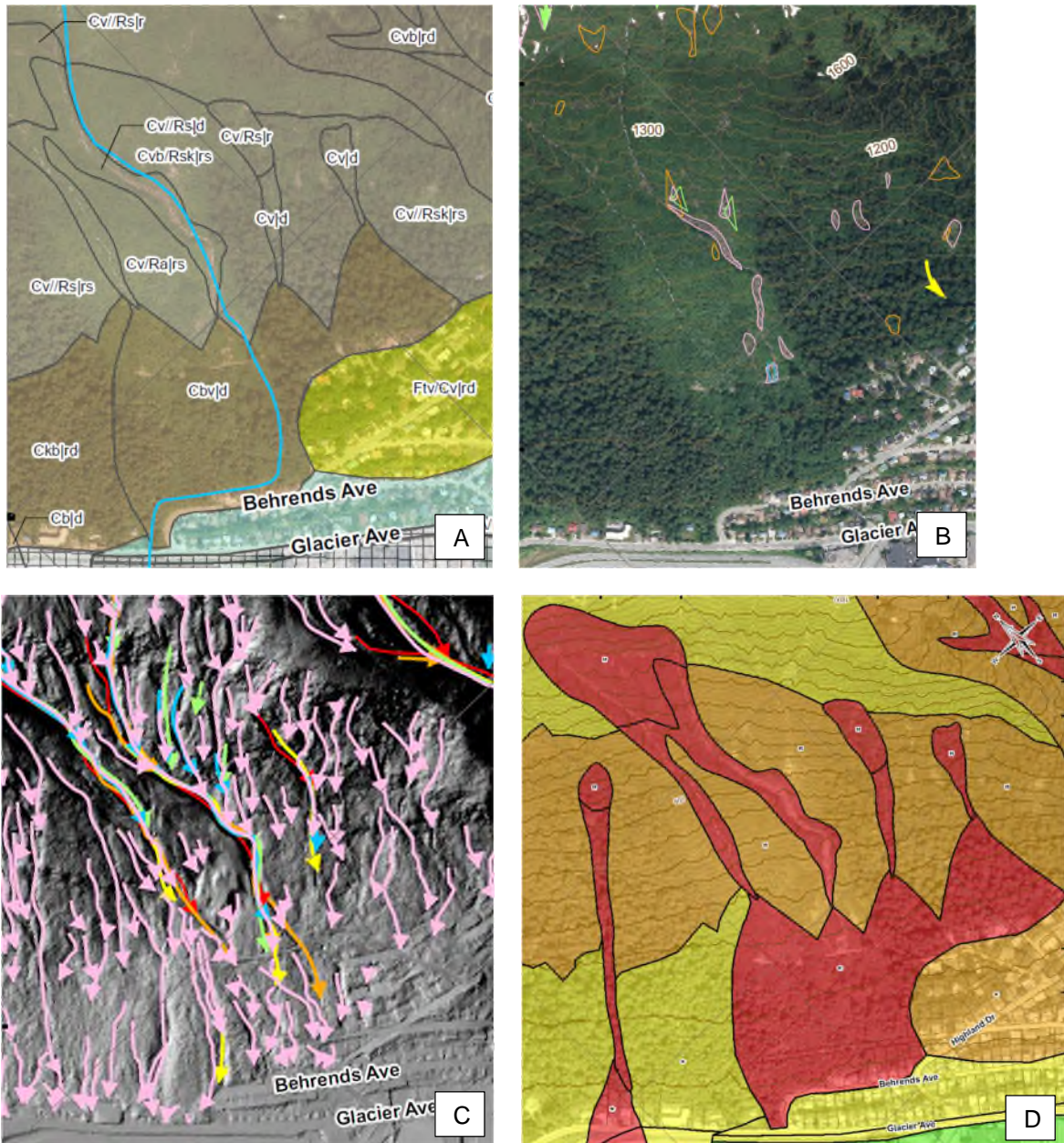


away from the site. Swanston (1990), understandably, stated that the “results and recommendations derived from more detailed site investigations by competent, licensed engineering geologists or geotechnical engineers” should govern.

Knowing what we know now, however, it is important to remember that a debris flow can occur *anywhere* on a colluvial cone, not just in the current incised channel. Furthermore, existing structures and even very large trees do not reduce the hazard; instead, trees and structures are often simply broken or crushed and entrained into the debris. It is important to recognize the true hazard represented by the debris flow terrain, and the *Severe* hazard designation was not arrived at lightly.

9. **Question/Comment:** The [west] side of the *Severe* hazard zone is noted to include a large area of colluvium in the Irwin Street to Highlands area, despite no recorded slide history since houses were built there. If the Swanston (1972) study is intended to support the *Severe* designation, maps and quotes from that study should be included in the report.

**Response:** The basis for the *Severe* hazard designation is not Swanston’s report, but rather Tetra Tech’s independent determination of the characteristics of the terrain units based on surficial geology mapping, historical air photo record analysis of slope movement features and gully erosion features, as well as incident reports and field observations. See also responses to Items 1 and 4 above. Specific to the *Severe* areas mapped in this part of the study area (Figure 1.6b and 1.6c), these are all colluvial areas (Figure 1.3b) that receive debris from several active gullies upslope (Figure 1.5b), including the previously mentioned Bathe Creek gully (see Figure 2 above), and several active gullies above Behrends Avenue and the west end of Highland Drive (see Figure 3 below). Several of the two dozen debris slides that occurred in various years in the immediate map area (Figure 1.4b) happened within gullies (compare Figures 3B and 3C below). The excerpt from Figure 1.4b (Figure 3B below) is also a good example of an avalanche track (area with no trees) that does not correspond perfectly with landslide activity and location.



**Figure 3: Excerpts from Figure 1.3b Surficial Geology (Figure 3A) and Figure 1.4b Historical Air Photo Record – Slope Movement Features (1948-2020) (Figure 3B), and excerpts from Figure 1.5b Historical Air Photo Record and LiDAR Data Analysis – Gully Erosion Features (1948-2013) (Figure 3C) and Figure 1.6c Landslide Hazard Designation Mapping (Figure 3D).**

While it is possible that houses have stood in one location for many years without being affected by a landslide, it does not mean there is nothing going on above them on the slope. If that activity has not reached a particular house yet, the resident may just have been lucky so far.

These are hazard maps, which indicate areas that are potentially hazardous. If there was a lot of potentially hazardous geomorphic process activity on a slope, or if new activity was identified in the field, that area was mapped as having a Severe hazard. For instance, debris could be building up on the slope directly above a house, or in a location where debris can potentially run towards a house (see Figure 4 below), and where it could become a more serious hazard in the future. Smaller debris slides and debris flows tend to accumulate debris and store it in wedges within gullies. Eventually, when a critical level of debris is reached, or a rainstorm of a particular size occurs, all that stored debris is scoured out of the gully, potentially resulting in a very large debris flow event. Similar events can occur on open slopes where slide debris piles up in lobes over days, months, or years, sometimes separated by channels of faster-flowing debris. These debris lobes can slowly be creeping downslope, until the critical moment when there is enough mass and enough water to make the debris flow rapidly downslope.



**Figure 4: Looking down towards the Gold Creek Flume Trail from the Bathe Creek debris flow gully. Note the large amount of debris in and alongside the gully, as well as the scarring on the tree beside the road that extends to at least 6 feet above ground surface. This gully is very active, and the debris that is deposited at the road crossing is regularly cleaned up. This road crossing has the potential to divert debris flows into the residential community downslope and to the west, especially if more than one surge of debris occurs before the debris can be cleaned.**

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## 4.0 CLOSURE

We trust this technical memo meets your present requirements. If you have any questions or comments, please contact the undersigned.

Respectfully submitted,  
Tetra Tech Canada Inc.



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File: 704-ENG.EARC03168-02A. April 27, 2022.

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File: 704-ENG.EARC03168-02A. April 27, 2022.

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Unless stipulated in the report, TETRA TECH has not been retained to investigate, address or consider and has not investigated, addressed or considered any environmental or regulatory issues associated with development on the subject site.

## 1.8 NATURE AND EXACTNESS OF SOIL AND ROCK DESCRIPTIONS

Classification and identification of soils and rocks are based upon commonly accepted systems and methods employed in professional geotechnical practice. This report contains descriptions of the systems and methods used. Where deviations from the system or method prevail, they are specifically mentioned.

Classification and identification of geological units are judgmental in nature as to both type and condition. TETRA TECH does not warrant conditions represented herein as exact, but infers accuracy only to the extent that is common in practice.

Where subsurface conditions encountered during development are different from those described in this report, qualified geotechnical personnel should revisit the site and review recommendations in light of the actual conditions encountered.

## 1.9 LOGS OF TESTHOLES

The testhole logs are a compilation of conditions and classification of soils and rocks as obtained from field observations and laboratory testing of selected samples. Soil and rock zones have been interpreted. Change from one geological zone to the other, indicated on the logs as a distinct line, can be, in fact, transitional. The extent of transition is interpretive. Any circumstance which requires precise definition of soil or rock zone transition elevations may require further investigation and review.

## 1.10 STRATIGRAPHIC AND GEOLOGICAL INFORMATION

The stratigraphic and geological information indicated on drawings contained in this report are inferred from logs of test holes and/or soil/rock exposures. Stratigraphy is known only at the locations of the test hole or exposure. Actual geology and stratigraphy between test holes and/or exposures may vary from that shown on these drawings. Natural variations in geological conditions are inherent and are a function of the historic environment. TETRA TECH does not represent the conditions illustrated as exact but recognizes that variations will exist. Where knowledge of more precise locations of geological units is necessary, additional investigation and review may be necessary.

## 1.11 PROTECTION OF EXPOSED GROUND

Excavation and construction operations expose geological materials to climatic elements (freeze/thaw, wet/dry) and/or mechanical disturbance which can cause severe deterioration. Unless otherwise specifically indicated in this report, the walls and floors of excavations must be protected from the elements, particularly moisture, desiccation, frost action and construction traffic.

## 1.12 SUPPORT OF ADJACENT GROUND AND STRUCTURES

Unless otherwise specifically advised, support of ground and structures adjacent to the anticipated construction and preservation of adjacent ground and structures from the adverse impact of construction activity is required.

## 1.13 INFLUENCE OF CONSTRUCTION ACTIVITY

There is a direct correlation between construction activity and structural performance of adjacent buildings and other installations. The influence of all anticipated construction activities should be considered by the contractor, owner, architect and prime engineer in consultation with a geotechnical engineer when the final design and construction techniques are known.

## 1.14 OBSERVATIONS DURING CONSTRUCTION

Because of the nature of geological deposits, the judgmental nature of geotechnical engineering, as well as the potential of adverse circumstances arising from construction activity, observations during site preparation, excavation and construction should be carried out by a geotechnical engineer. These observations may then serve as the basis for confirmation and/or alteration of geotechnical recommendations or design guidelines presented herein.

## 1.15 DRAINAGE SYSTEMS

Where temporary or permanent drainage systems are installed within or around a structure, the systems which will be installed must protect the structure from loss of ground due to internal erosion and must be designed so as to assure continued performance of the drains. Specific design detail of such systems should be developed or reviewed by the geotechnical engineer. Unless otherwise specified, it is a condition of this report that effective temporary and permanent drainage systems are required and that they must be considered in relation to project purpose and function.

## 1.16 BEARING CAPACITY

Design bearing capacities, loads and allowable stresses quoted in this report relate to a specific soil or rock type and condition. Construction activity and environmental circumstances can materially change the condition of soil or rock. The elevation at which a soil or rock type occurs is variable. It is a requirement of this report that structural elements be founded in and/or upon geological materials of the type and in the condition assumed. Sufficient observations should be made by qualified geotechnical personnel during construction to assure that the soil and/or rock conditions assumed in this report in fact exist at the site.

## 1.17 SAMPLES

TETRA TECH will retain all soil and rock samples for 30 days after this report is issued. Further storage or transfer of samples can be made at the Client's expense upon written request, otherwise samples will be discarded.





<b>To:</b>	Teri Camery (CBJ)	<b>Date:</b>	April 27, 2022
<b>c:</b>	Scott Ciambor (CBJ)	<b>Memo No.:</b>	3
<b>From:</b>	Rita Kors-Olthof, Vladislav Roujanski, Shirley McCuaig	<b>File:</b>	704-ENG.EARC03168-01 / 704-ENG.EARC0168-02A
<b>Subject:</b>	Mapping Overview at Starr Hill Subdivision and Additional Information Downtown Juneau Landslide and Avalanche Hazard Assessment		

## 1.0 INTRODUCTION

This technical memo addresses some of the comments and questions that arose from Tetra Tech Canada Inc.'s (Tetra Tech) Issued-for-Review (3<sup>rd</sup> Draft) Report, Downtown Juneau Landslide and Avalanche Assessment, dated May 28, 2021 (Tetra Tech 2021), and the Landslide and Avalanche Hazard Public Meeting that took place on July 21, 2021.

The City and Borough of Juneau (CBJ) has requested a response for each of three key points, as described in CBJ's email dated July 27, 2021. Two of these items were addressed in Technical Memos #1 and #2, which have since been updated (in Appendix C of the main report; Tetra Tech 2022a, 2022b). This Technical Memo #3 responds to commentary and requests for additional information about hazards surrounding the Starr Hill subdivision. It includes the information provided in the Issued-for-Review memo dated September 16, 2021, as well as the supplementary information for Question #5 provided by email on September 17, 2021, as well as some additional mapping information compiled since April 1, 2022. A few additional remarks have also been provided for Question #14.

## 2.0 LANDSLIDE HAZARD DESIGNATIONS AND BOUNDARIES

### 2.1 Mapping Overview at Starr Hill Subdivision

#### 2.1.1 Comparing Adopted (1987) and Proposed (2021) Mapping

The residents of the Starr Hill Subdivision have been concerned to discover that landslide hazards designated *High* and *Severe* have been identified on the slopes around the subdivision. These hazards had not been identified in the 1987 adopted hazard mapping, in which only *Moderate* hazards had been shown at existing structures along most of the adjacent portion of Basin Road, 6<sup>th</sup> Street, Nelson Street, and several houses on 5<sup>th</sup> Street and in a zone upslope of 5<sup>th</sup> Street beyond the built roads of Kennedy and East Streets, as shown in Figure 1A below. Only a few structures further upslope of Kennedy, East, and Harris Streets were already mapped within a *Severe* zone in 1987, although continuing south and then southeast onto the main slope of Mt. Roberts, many more structures were mapped within *Severe*.

As seen in Figure 1 below, the main difference between the 1987 adopted hazard mapping and the new 2021 proposed hazard mapping is that many of the former *Moderate* areas surrounding Starr Hill (colored in pale lavender on Figure 1A) are now designated *High* or *Severe* (colored in dark pink on Figure 1B).

The 1987 adopted hazard mapping is understood to have been based on Swanston (1972). However, the 1987 mapping follows property lines, resulting in numerous right-angle corners in the hazard boundaries. The 2021 mapping does *not* follow property lines. It follows hazard designations based on terrain features, such as surficial geology, geomorphology (evidence of past and more recent slope movement types accentuated by vegetation patterns), and activity levels for slope instabilities as seen from the air photos and confirmed by field observations. For instance, along 6<sup>th</sup> Street, between East Street and Nelson Street, the lower boundary of the new 2021 proposed hazard zone does *not* arbitrarily stop at the upslope property lines of the affected properties as it does in the 1987 mapping. The same is true upslope of Nelson Street, and upslope of 5<sup>th</sup> Street, where the lower boundary of the 2021 proposed hazard mapping does *not* arbitrarily stop at the upslope property lines of the affected properties.



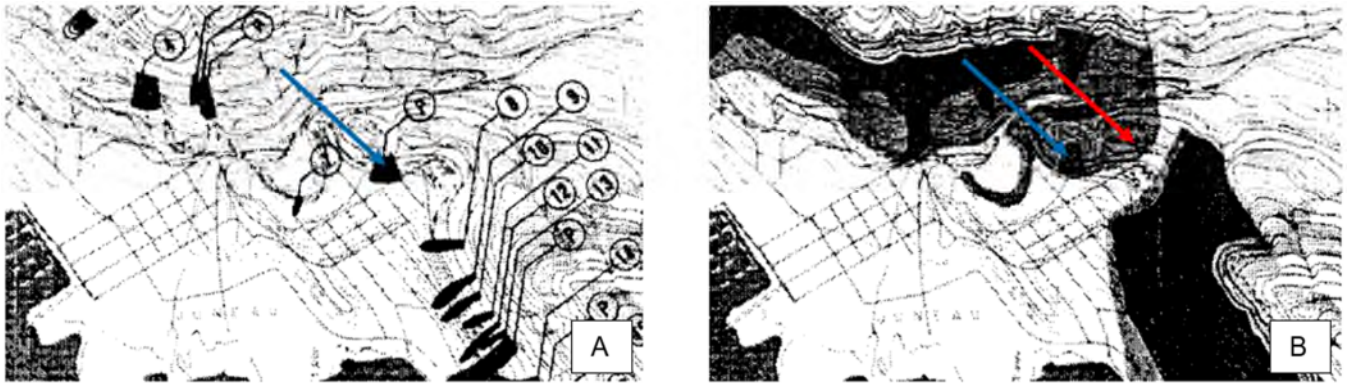
**Figure 1: Excerpts from CBJ’s 1987 adopted hazard mapping (Figure 1A) and comparison of 1987 adopted hazard mapping and 2021 proposed hazard mapping (Figure 1B) at the Starr Hill subdivision.**

Since Tetra Tech has identified some important differences in the mapping methodology seen in the 1987 adopted mapping and Tetra Tech’s 2021 mapping, it is useful to show the origins of that 1987 mapping, as discussed further in Section 2.1.2.

### 2.1.2 Comparing Swanston (1972) and Tetra Tech’s (2021) Mapping

This section has some general comments comparing the mapping from Swanston (1972) and Tetra Tech (2021), followed by a few comments for specific slope sections above Starr Hill.

Swanston (1972) did not create arbitrary transitions in hazard designation based on property lines. Despite the poor scan quality of this old report, it is clear that Swanston based his 1972 hazard designations on geology and landslide features, *not* on property lines, as seen below in Figure 2.

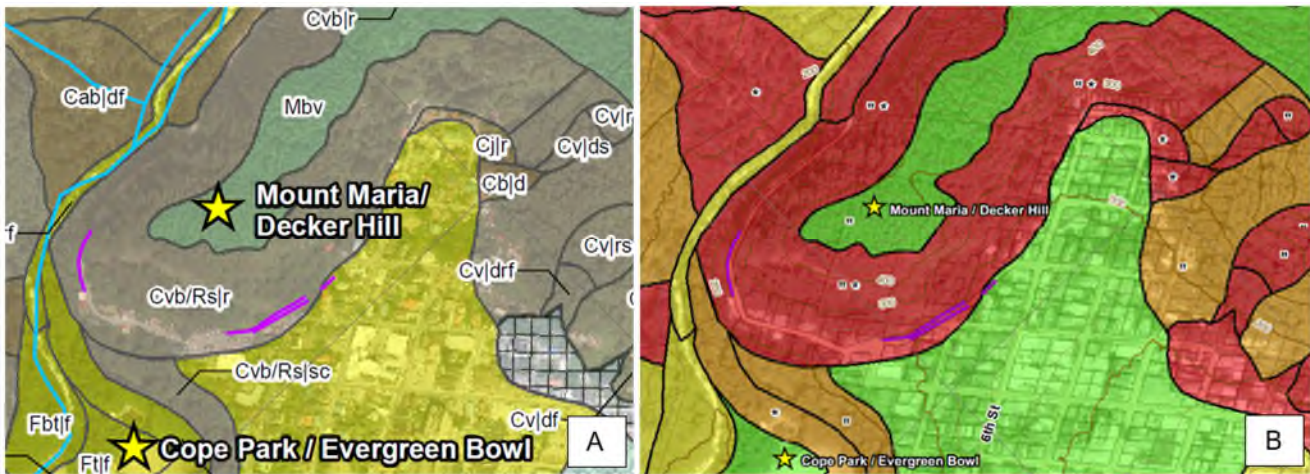


**Figure 2: Excerpts from Swanston (1972) – Figure 5 – Historic Landslide Deposits (Figure 2A) and Figure 6 – Mass Wasting Hazard Areas (Figure 2B).**

In Swanston’s Figure 5, “P” means “Prehistoric” (before 1880). A prehistoric slide area was noted on the south side of Mt. Maria (blue arrow in Figure 2A). In Swanston’s Figure 6 (Figure 2B), hazards for mass wasting (landslide) hazard areas are shown, with *Potential* Hazards marked in gray, and *High* Hazards marked in black. The prehistoric slide is shown as a *High* Hazard area (blue arrow in Figure 2B). A small cliff with a talus deposit above the corner of 6<sup>th</sup> and Nelson Streets was also designated as being in a *High* Hazard area (red arrow in Figure 2B).

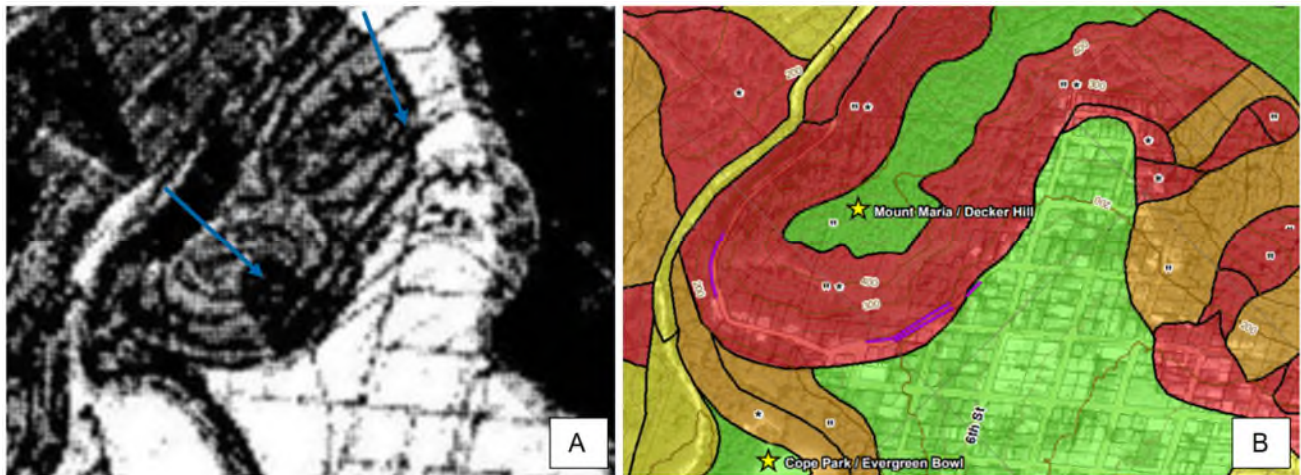
Comparing Swanston’s hazard mapping (Figure 2B) with the 1987 adopted mapping (Figure 1A), CBJ renamed Swanston’s “*Potential* Hazard” as *Moderate* Hazard and renamed Swanston’s “*High* Hazard” as *Severe* Hazard.

A summary of Tetra Tech’s mapping of the Starr Hill area and the adjacent portion of Basin Road is shown below in Figure 3, with surficial geology in Figure 3A and landslide hazard designation mapping in Figure 3B. There is a clear correlation between the shapes of the surficial geology units and the associated landslide hazard designations.



**Figure 3: Excerpts from Tetra Tech Figure 1.3b Surficial Geology (Figure 3A), and Figures 1.6c and 1.6h Landslide Hazard Designation Mapping (Figure 3B).**

Some parallels can be seen in the locations of the mapping boundaries of Swanston (1972) and Tetra Tech (2021), in that they appear to follow the terrain (in contrast to the 1987 mapping which defers to nearby property lines). However, there are some significant differences in the details, due to better quality data, i.e., higher resolution imagery and new LiDAR data available to Tetra Tech, and more advanced mapping techniques used in the current study (Figure 4).



**Figure 4: Comparison of Swanston’s Figure 6 (Figure 4A) and Tetra Tech’s Figures 1.6c and 1.6h Landslide Hazard Designation Mapping (Figure 4B). Swanston (1972) uses *Potential Hazard* (gray) and *High Hazard* (black), designated in the 1987 adopted mapping as *Moderate* and *Severe* hazards. Tetra Tech uses *Low* (green), *Moderate* (yellow), *High* (orange), and *Severe* (red) hazard designations.**

As shown in Figure 4A, Swanston mapped most of Mt. Maria on the Starr Hill side as having a *Potential Hazard* (gray area), except at the prehistoric slide area and a small cliff above the corner of 6<sup>th</sup> and Nelson Streets, which were designated as being in *High Hazard* areas (blue arrows). These two areas are also shown with blue arrows in Figure 5. Heading northeast along 6<sup>th</sup> Street from Basin Road towards Nelson Street, the lower boundary of Swanston’s *Potential Hazard* area was mapped progressively closer to 6<sup>th</sup> Street.

Along the entire southeast edge of Starr Hill, including a portion of the mapped lots, Swanston (1972) marked an area of *Potential Hazard*, with *High Hazard* marked further upslope on Mt. Roberts. The lower boundary of Swanston’s *Potential Hazard* area begins at about Harris and 4<sup>th</sup> Streets, becoming progressively closer to 5<sup>th</sup> Street heading northeast. The northeast corner of Swanston’s mapped landslide hazard area curves to the north to encompass the terrain southeast and northeast, upslope of the corner of Nelson and 5<sup>th</sup> Streets. Swanston did not designate hazards along or above the northwest part of Nelson Street. CBJ’s 1987 adopted map called that area *Moderate* (Figure 1A).

The sections that follow provide some additional details about the slope sections for which hazard designation changes have been proposed.

### **Nelson Street (between 5<sup>th</sup> and 6<sup>th</sup> Streets)**

Swanston’s mapped rockslide hazard appears to continue around the corner to Nelson Street (more red arrows in Figure 5), although that is less clear due to the poor scan quality of Swanston’s mapping. The presence of rockfall/rockslide hazard areas along Nelson Street did not appear to be reflected in Swanston’s hazard mapping (Figure 4A).

The findings in Swanston (1972) are generally consistent with Tetra Tech’s findings on the northwest and northeast sides of the Starr Hill Subdivision, except for the gap in Swanston’s hazard mapping along Nelson Street.

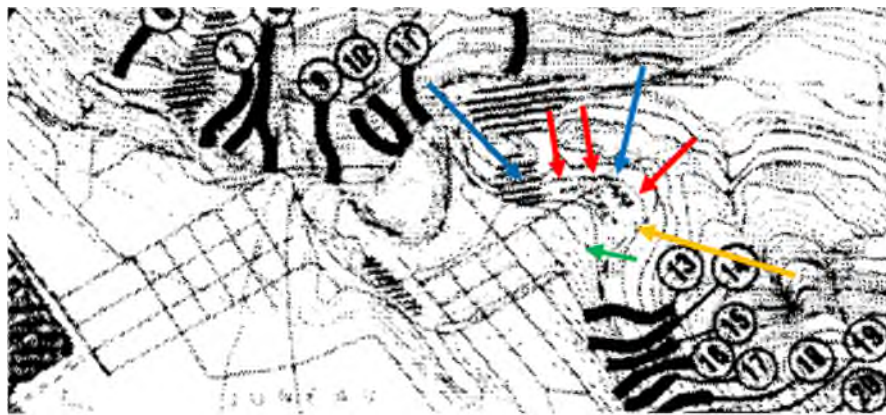
### **Corner of Nelson Street and 5<sup>th</sup> Street to Harris Street**

Tetra Tech identified some additional debris slide and debris flow features on the southeast side of the Starr Hill Subdivision that were not specifically identified by Swanston, but which do appear to account for Swanston’s overall hazard designations for that slope (Figure 2B; Figure 4A).

## Debris Flows above 5<sup>th</sup> Street between Park and Kennedy Streets and Kennedy and East Streets

On the southeast side of the subdivision, Swanston (1972) also found a record of a “slump” that occurred on November 27, 1935, at “5<sup>th</sup> Street above Kennedy” Street that did not cause damage. Swanston did not map that feature; however, nor did he detail the active slope processes at that location. The debris flow feature identified by Tetra Tech between Kennedy and Park Streets and upslope (southeast) of 5<sup>th</sup> Street, might be related to the “slump” mentioned by Swanston (located at approximately the orange arrow on Figure 5). That significant debris flow gully (called G000 Park in the new avalanche mapping) appears to be a different debris flow than the feature identified by Miller (1975), which is located slightly to the southwest, between East and Kennedy Streets (green arrow on Figure 5).

At that location, Miller (1975) showed a debris flow about 85 yards long that he reported as a 1972 event. Due to this location being located at the edge of one of the air photos, and possibly due to regrowth of vegetation since 1972, Tetra Tech’s review of the 1977 air photos was inconclusive. However, Google Street View (July 2011 imagery), near the southeast end of the paved part of East Street, clearly shows the aftermath of a recent debris flow and/or erosion type event from upslope of the road at 415 East Street. The 2013 imagery also suggests disturbed ground between the house closest to the road and the next house located almost due east at 622 4<sup>th</sup> Street.



**Figure 5: Excerpt from Swanston (1972) Figure 7 – “Mass Wasting Channels and Rock Slide Areas.” Striped map areas are rockslide hazard areas; heavy numbered lines are mass movement channels.**

### 2.1.3 Reasons to Update the Landslide Hazard Designations around Starr Hill

Swanston (1972) *specifically* identified many of the rockfall/rockslide hazards on the Basin Road and 6<sup>th</sup> Street boundaries of Mt. Maria, and along Nelson Street, and *generally* identified landslide hazards between Nelson Street and Harris Street above 5<sup>th</sup> Street, near the northwest end of the Mt. Roberts ridge.

However, Swanston’s hazard designation system was slightly less conservative than Tetra Tech’s designation system. This difference is partly due to Tetra Tech’s modern mapping capabilities identifying more features than might have been detected in 1972, particularly on the southeast side of the subdivision, and partly due to the Four-Tier Landslide Hazard Designation System developed by Tetra Tech.

The other main difference in hazard designation mapping is that Swanston’s hazard mapping between Nelson Street and Harris Street on the southeast side of Starr Hill shows a lower level of hazard activity along the toe of slope than upslope. Swanston’s mapping transition; however, does *not* entirely reflect the actual landslide hazards in the area. For example, landslide hazard is associated with the *deposition zone* of landslide debris just as much as it is with the initiation zone or the path of the debris. If the residences are located within the natural deposition

zone, then, logically, damage *could* occur if a landslide does happen. Therefore, the *Severe* hazard designations *should* extend into the deposition zones along Basin Road, 6<sup>th</sup> Street, Nelson Street, and around the corner along and above 5<sup>th</sup> Street. Similarly, the debris flow deposition zone between Park and Kennedy Streets widens towards the toe of slope, due to the way that debris flow cones or fans are formed (as explained in Technical Memo #2). The deposition zone of the debris flow gully (the lower cone/fan-shaped area) *should* also be designated as a *Severe* hazard.

Tetra Tech has mapped the slope in the vicinity of 415 East Street and 622 4<sup>th</sup> Street as being within a *High* landslide hazard designation zone. However, given the findings of an apparent debris flow feature at that location, the judgement of whether to upgrade the landslide hazard designation to *Severe* should be made after a site-specific investigation in that area. It does appear possible that the debris flow might be related to a cutline upslope (apparently a former powerline alignment), and the problem might be solved by remediating the surface water drainage at the cutline. However, if the feature is not related to water drainage problems originating at the cutline, this area should be mapped as *Severe*.

Based on Tetra Tech's mapping of surficial geology, slope movement activity, gully erosion features, and field observations, as well as some recent landslide events documented in the past 10 to 12 years, it appears that much of the *Moderate* hazard terrain in this area should be reclassified to *High* or *Severe* hazard, as was done in the 2021 hazard designation mapping. *Furthermore, arbitrary hazard boundaries along property lines should be removed as not reflecting the true threat to the public safety, i.e., hazard designations based on property lines do not adequately describe the hazards.*

*Severe* hazard designations are assigned to the areas subject to rockfall, debris slides, and debris flows, as shown on the surficial geology map in Figure 3A. Areas with a *High* hazard rating were assigned based on the results of the semi-quantitative analysis. These areas are expected to experience rockfall that damages but does not always knock out trees, and as such are a less severe hazard than a debris flow or debris slide that removes everything in its path. Evidence of this type of rockfall activity was identified during the field investigation.

## 2.2 Requests for Additional Information for Starr Hill Subdivision

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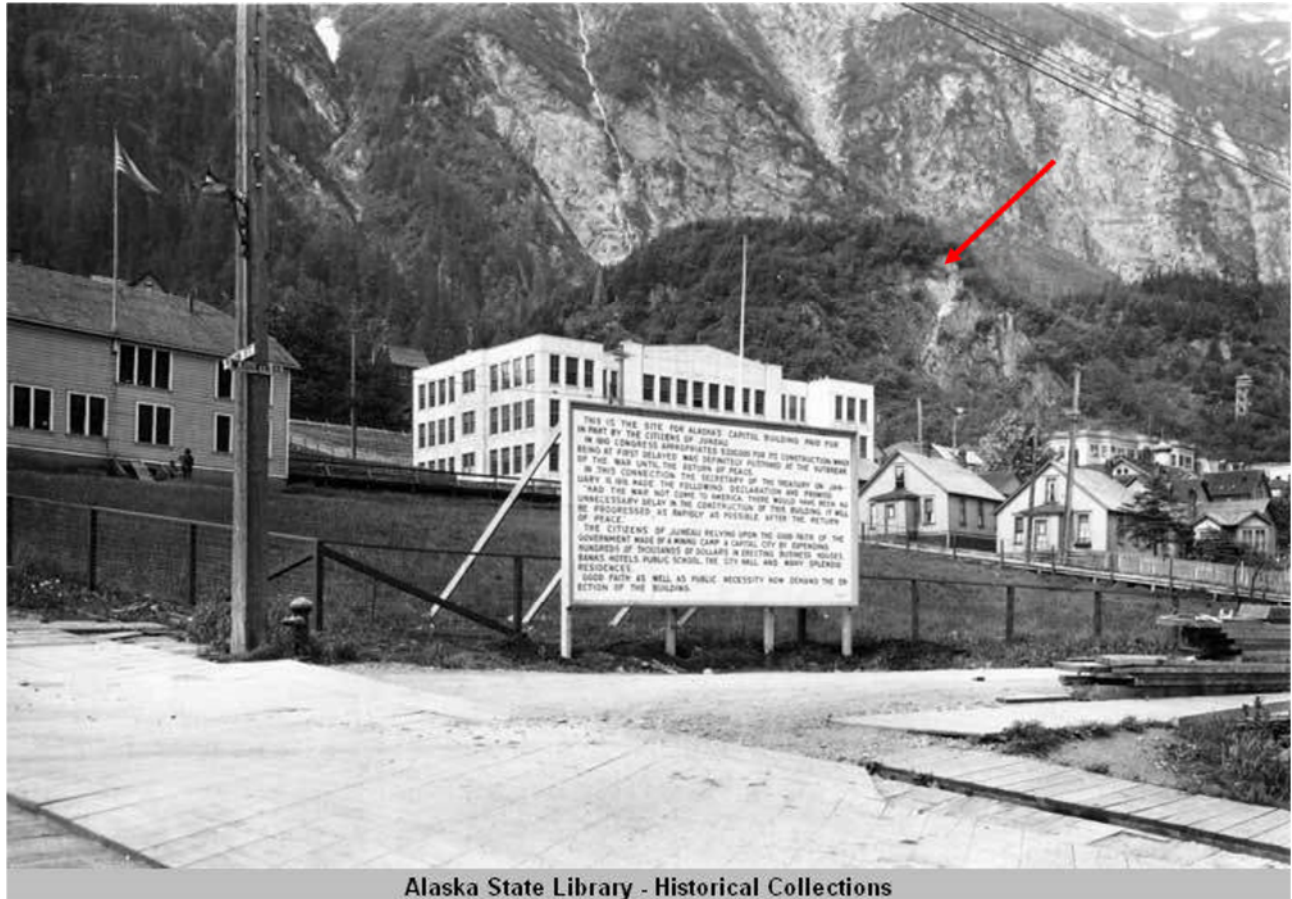
Numerous comments and questions were received from residents about the Starr Hill mapping area. These comments and questions have been excerpted and documented below in a question-and-answer format. In cases where questions were similar or related, these have been combined for the response. Tetra Tech has also incorporated and greatly appreciates several anecdotal observations and photos from Starr Hill residents that provide additional context for the slopes around the subdivision.

1. **Question/Comment:** I've seen the rockfall above Basin Road, and I can see why that slope is in a *Severe* hazard zone, but what about the slope above 6<sup>th</sup> Street? Why is that *Severe*?

**Answer:** Let's start with the work done by Swanston (1972) to understand why that is. Along the south side of Mt. Maria, Swanston (1972) identified rockfall/rockslide hazards primarily at the prehistoric rockslide and at the corner of 6<sup>th</sup> and Nelson Streets where a talus deposit was observed (blue arrows in Figure 5). Swanston also reported small deposits of angular rock fragments and talus above 6<sup>th</sup> Street from Basin Road to Nelson Street, which apparently correspond to the rockfall/rockslide hazard mapped above 6<sup>th</sup> Street (red arrows point to the hazard in Figure 5). These observations were confirmed by Tetra Tech's fieldwork, during which numerous unstable rock cliffs and bluffs were also observed above 6<sup>th</sup> Street (Figures 6 and 7). Swanston (1972) further noted that, although the bedrock dips into Last Chance Basin (on the north side of Mt. Maria), freeze-thaw action in the fractures and joints of the exposed bedrock, and water acting as a lubricant in the cracks, result in instabilities. The elevated level of slope movement activity on this slope, including several well-established slide

paths below prominent bedrock bluffs and cliffs, requires the slopes below the cliffs to be designated as *Severe* hazard. These are the kinds of processes that have been ongoing since long before Swanston's observations and are expected to continue.

The prehistoric slide area (Figure 5, blue arrow on left side) appears to be in the same place where a rockfall/rockslide was reported on October 18, 1913 (Swanston 1972, Figure 6), and where a distinct landslide scar is still present despite reforestation of the slope (Figure 13). Swanston reported that several houses had been destroyed, and that a deposit of angular rock fragments had been created above Basin Road between 6<sup>th</sup> and 7<sup>th</sup> Streets.



**Figure 6: Looking north towards the Juneau Public School. Rockfall/rockslide scar on Decker Hill/Mt. Maria in the background was the likely origin of the October 18, 1913 landslide event (red arrow). The rock slope immediately to the right of the red arrow appeared more active (less reforestation) than the slope to the left in 1918, and some of the debris might also have originated from there. See Figure 13 for present-day view of Mt. Maria. (Photo credit: Alaska State Library – Historical Collections, [ASL-Juneau-Capitol-Building-1](#), Alaska State Library Place File. Photographs. ASL, ca. 1918.)**

A news story about the 1913 event reported two large rocks, each several tons in weight, falling from the cliff on the side of Decker Hill facing town and above Basin Road (Figure 6). One of these rocks impacted a huge boulder below, which had lain there for many years and, although the falling rock lifted the existing boulder up on edge, it was prevented from travelling further downslope. That existing boulder was reported to be located at the edge of the road, opposite the Nelson home, protecting it. The other large rock that fell was deflected and crossed the road to crush a woodshed below Basin Road, at the Price home. The power poles along Basin Road were also snapped, resulting in a short circuit (The Alaska Daily Empire October 20, 1913). The Nelson

home is likely Structure C-8 Nelson House II, located downslope of Basin Road at the northeast corner of Harris Street and 6<sup>th</sup> Street, as shown by the purple arrow on Figure 12 (CBJ 1986). The Price home was not listed in the inventory, so its exact location is not known.

To summarize the results of Tetra Tech’s mapping and fieldwork upslope of 6<sup>th</sup> Street and Nelson Street and continuing along Basin Road on the slopes of Mt. Maria/Decker Hill, there are unstable bedrock bluffs that are considered a *Severe* hazard due to observed features and potential rockfall activity that comes close to or into residential areas (Figures 7 and 8). There is also a talus deposit at the corner of 6<sup>th</sup> and Nelson Streets (Figure 8D). Exposed talus means that there is still rockfall coming down from above. If the rockfall activity had ceased, there would be much more regrowth of vegetation than is apparent now, nearly 50 years after Swanston first described the talus deposit. Above the houses in the rockfall area, trees show damage from being hit by large angular boulders, many of which are so large they could easily injure or kill a person who happened to be in their paths (Figure 7). Along the houses on 6<sup>th</sup> Street, the *Severe* zone affects the backyard, but might or might not affect the house. Figure 8 shows examples of rock bluffs, slide tracks, and talus. The housing itself obscures the effects of the rockfall activity – if a boulder rolls into a yard or onto the road, it is typically removed, so the evidence is no longer available for mapping. The boundary of the *Severe* hazard area for Mt. Maria is thus very conservative and may well extend further southeast than that shown.

See also Question #3 below for more information on what the rockfall/rockslide paths look like on this slope.



**Figure 7: Photos from Tetra Tech’s main report, at the southwest end of Mt. Maria above Basin Road at Harris and 7<sup>th</sup> Street. Photo P24 (Figure 7A) shows a tree damaged by rockfall. Photo P25 (Figure 7B) shows the typical size of some of the fallen rocks.**





**Figure 8: Rockslide track and cliff above rockfall at left-hand blue arrow on Figure 5 (Figure 8A); typical rock bluff above 6<sup>th</sup> Street at red arrows on Figure 5 (Figure 8B); rock cliff with detached blocks above corner of 6<sup>th</sup> Street and Nelson Street at right-hand blue arrow on Figure 5 (Figure 8C); and talus deposit below rock cliff, 6<sup>th</sup> Street visible below (Figure 8D).**

2. **Question/Comment:** Could you please explain the slope hazards above Nelson Street? I've noticed several fallen trees and continued evidence of mudslides in this area.

**Answer:** Upslope of Nelson Street, there are debris slides and rockfall (Figures 12 and 15). Due to additional information about landslides above Nelson Street and around the corner above 5<sup>th</sup> Street that was received during the public review process, the hazard in this area is now mapped as Severe (Figure 3B). Figure 9 shows some examples of active debris slide paths and rock bluffs above Nelson Street. See Question #3 for information on the effects of historical forestry activities on this slope and to see where the most prominent landslide paths are located. See Questions #5 and #6 for information on the slopes above the corner of Nelson and 5<sup>th</sup> Streets. See Question #12 for information on the possible effects of the old Mt. Roberts Trailhead.



**Figure 9: Active debris slide paths above Nelson Street are shown in Figures 9A and 9B; rock bluffs and broken or damaged trees above Nelson Street are shown in Figures 9C and 9D.**

3. **Question/Comment:** Starr Hill/Mt. Maria were clearcut early on, and the area is gradually becoming reforested. These historical conditions would have exaggerated the landslide risks and frequencies during that time, while reforested conditions should reduce the risks, even with climate change-driven precipitation events. There was no discussion about that in the report.

**Answer:** Residents provided two historical photos from 1901 and 1940 with perspectives close enough to the slopes for good comparisons to be made with more recent imagery. Tetra Tech has added another photo from 1902 showing Mt. Maria, two recent Google Earth oblique views to compare how the slopes appear now, and the 2013 LiDAR that clearly shows the bedrock outcrops on Mt. Maria.

It is important to recognize that the slopes around Starr Hill were not uniformly treed prior to clearcutting (Figures 10 through 14). Tetra Tech agrees that increased interception from reforestation would help to reduce the infiltration of surface water onto slopes that are sensitive to the input of additional water, particularly slopes with thin colluvium over bedrock. Equally important is the reduction of water that would otherwise flow from the reforested slopes – as surface water drainage or subsurface water drainage – into the swales and gullies that are (and always were) sparsely treed or lacking tree cover altogether. Reforestation would be expected to have some benefit to slope stability.

The evaluation of climate change effects was not in the project scope, so the effects of climate change on precipitation in Juneau are not known in any detail. If (as suspected) the likelihood of extreme precipitation and/or wind events is increasing due to climate change, reforestation is likely not enough to reduce slope instability hazards, especially on slopes with shallow bedrock. Comparing Tetra Tech's observations from 2019 to the observations from residents in 2021, landslide activity above Starr Hill is clearly ongoing.



Alaska State Library - Historical Collections

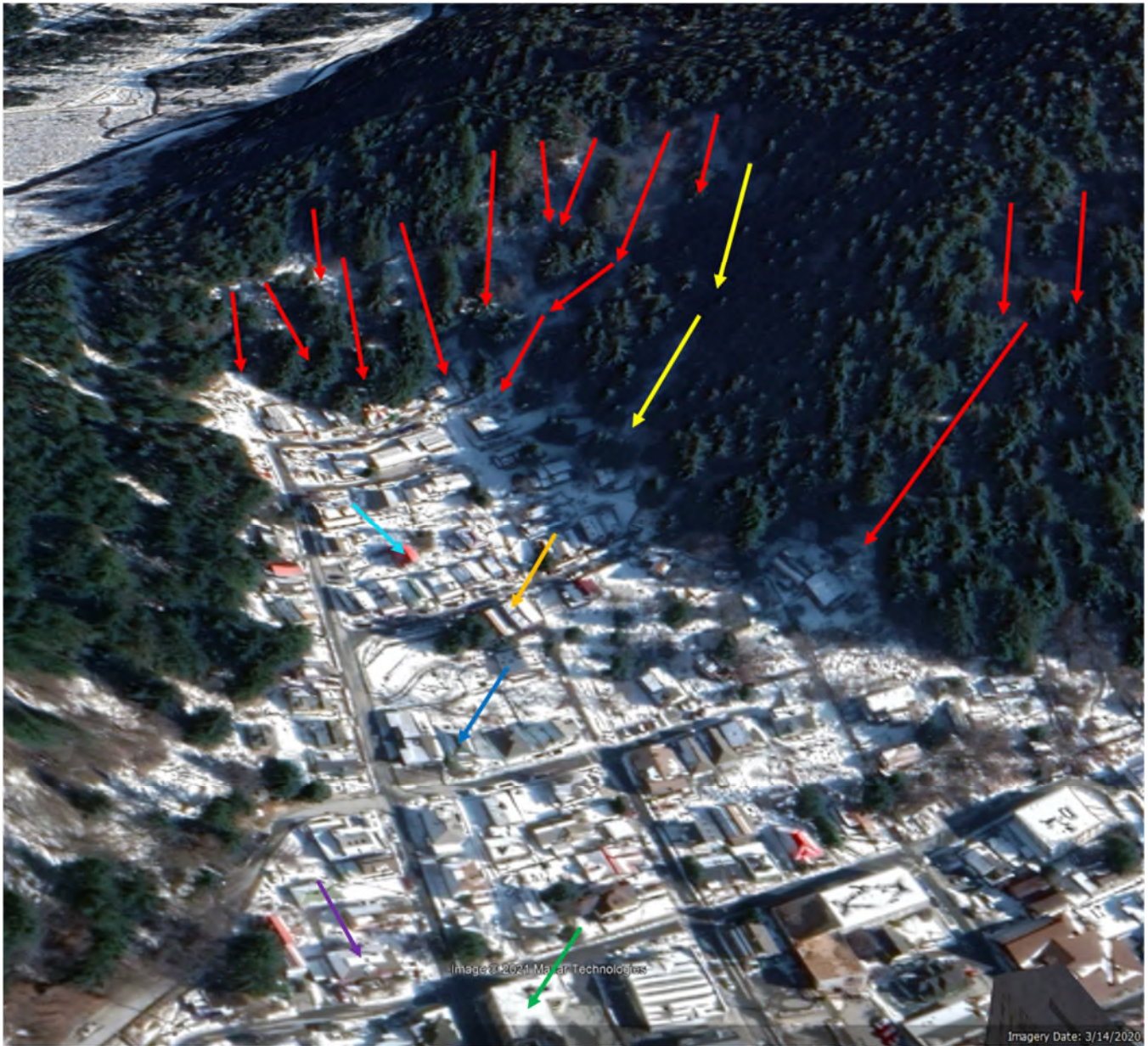
**Figure 10: Looking east-northeast toward Starr Hill from top of Chicken Ridge, 7<sup>th</sup> and Franklin Streets, July 1901. Slopes appear to have been recently clearcut (within the previous few years) with numerous stumps visible. There are some swales and slope sections with few or no stumps, indicating that this slope was not uniformly forested prior to clearcutting (red arrows). At least two recent soil debris slides are visible above what appears to have been Park Street in 1901. (Photo credit: Alaska State Library – Historical Collections, [ASL-P1-506](#), Vincent Soboleff Photograph Collection, ca. July 1901.)**

Landmarks shown in Figure 10 include the Distin-Dawes-Pelto House at 529 East Street (blue arrow), St. Ann's Hospital staff residence (green arrow) at the south corner of 6<sup>th</sup> and Harris Streets, apparent precursors to the Lund Houses I and II at 504 and 510 Kennedy Street (yellow arrow), and the Mitchell House at 715 – 6<sup>th</sup> Street (aqua arrow), based on the Inventory of Historic Sites and Structures (1986) and various historical photos from the Alaska State Library. Some of the dates and descriptions are uncertain in the inventory, and it is possible that changes or additions have been made to some of the structures that still exist. As well, some of the structures present in this photo no longer exist, or were replaced with other structures. Park Street appears to have been the approximate upper end of the developed area in 1901, since the earliest houses above Nelson Street were built in 1928.



**Figure 11: Looking east-northeast, this view of Starr Hill in 1940 is from a little further southwest of the view in Figure 10, and somewhat foreshortened. After more than 40 years of tree growth after clearcutting, the swales and gullies where no coniferous trees grow are especially clear in this photo. Selected swales and gullies are shown with red arrows; several other swales and gullies are also visible between the red arrows. (Photo credit unknown.)**

Landmarks shown in Figure 11 include the Distin-Dawes-Pelto House at 529 East Street (partly obscured, blue arrow); St. Ann's Hospital (green arrow) at the south corner of 6<sup>th</sup> and Harris Streets, with the newer concrete section northeast of the green arrow (replacing the former staff residence), and the older wooden section to the southwest; the Lund Houses I and II at 504 and 510 Kennedy Street (yellow arrow); and the Mitchell House at 715 – 6<sup>th</sup> Street (aqua arrow), based on the Inventory of Historic Sites and Structures (1986) and various historical photos from the Alaska State Library. Some of the dates and descriptions of structures are uncertain in the inventory, and it is possible that changes or additions have been made to some of the structures that still exist. As well, some of the structures present in this photo may no longer exist, or were replaced with other structures. Nelson Street was the upper end of the developed area in 1940, with most of the houses above Nelson Street built by then.



**Figure 12: Looking east-northeast, this Google Earth view is from an eye elevation of about 1,100 feet and at a snowy time of year (March 2020), so that the areas without coniferous trees can be seen more easily. The same swales and gullies that lack tree cover that were seen in the 1940 photo (Figure 11) are still visible in 2020, along with a few more. The landmarks are the same as in Figure 11, although St. Ann’s Hospital is now called St. Ann’s Center, and a new landmark at Nelson House II has been added below Basin Road (purple arrow). (Image credit: Google Earth 2022.)**

In Figure 12, one of the main gullies (shown with two bright yellow arrows, ending between Park and Kennedy Streets) is the G000 Park debris flow gully. See Figure 15 for more information about this gully.



**Alaska State Library - Historical Collections**

**Figure 13: Looking north toward Starr Hill from the waterfront. Mt. Juneau is in the background. Slopes appear to have been recently clear-cut with numerous stumps visible above the buildings. The exposed rock slope on the west end of Mt. Maria (Decker Hill) is the location of the pre-historic rockslide, where in 1913 another major rockfall event would occur. (Photo credit: Alaska State Library – Historical Collections, [ASL-P334-16](#), Ark A. Tower Photograph Collection, June 8, 1902.)**

The perspective of the 1902 photo (Figure 13) makes it more difficult than the 1901 photo (Figure 10) to see the slope details, and apparent logging debris remaining above the upper end of 6<sup>th</sup> Street also obscures the slope. However, there are some swales and slope sections with few or no stumps visible, indicating that the Mt. Maria slope was not uniformly forested prior to clearcutting (red arrows). Above 6<sup>th</sup> Street, areas lacking forest cover appear to be associated with the rock bluffs visible upslope. Landmarks include 529 East Street at the blue arrow, St. Ann's Hospital staff residence (green arrow). Just to the southeast along 5<sup>th</sup> Street (in the foreground of the staff residence) were the Church of the Nativity, the chancellery, and St. Ann's Hospital (later the school, and now the Parish Hall, orange arrow), based on the Inventory of Historic Sites and Structures (1986) and various historical photos from the Alaska State Library.



**Figure 14: Looking north-northwest, this Google Earth view is from an eye elevation of about 1,100 feet in early spring (April 2020), so that the slopes currently lacking coniferous trees can be seen more easily. Several of the swales and slope sections lacking tree cover that were seen in the 1902 photo (Figure 13) are still visible in 2020, along with a few more slide paths (red arrows). The apparent alignment of the paths lacking trees is slightly different in Figures 13 and 14, due to the different perspectives of the images. The landmarks in Figure 14 are the same as those in Figure 12. (Image credit: Google Earth 2022.)**

At the top ends of all the paths are rock bluffs or cliffs (Figure 14), which are the source of the rockfalls and rockslides that periodically scour out lower-growing vegetation along the paths, or damage mature trees alongside the paths if the debris is large enough. A couple of the paths seen in 1902 between Kennedy and Park Streets are obscured by trees in 2020 but are likely still present under the tree canopy. Some of the swales that seem to end mid-slope in this image likely continue further downslope under the tree canopy. In some areas of the slope, there are still some trees below rock bluffs, but they tend to be smaller than the trees on slopes not regularly affected by rockfall or rockslide debris (Figure 7). On the far left, the path shown crossing Basin Road represents the rock debris that impacted 712 Basin Road about 12 years ago (CBJ 2020).

Three cutlines are visible in Figure 14: on the left, the powerline above Basin Road; in the middle, where an old cutline crosses over to the north side of Mt. Maria; and, on the right, parallel to 6<sup>th</sup> Street, another old cutline crosses above the corner of 6<sup>th</sup> and Nelson Streets into Last Chance Basin. The latter two cutlines may be related to old mining infrastructure, and/or powerlines.





**Figure 15:** This excerpt from the 2013 LiDAR image shows several bedrock cliffs and bluffs above Basin Road, 6<sup>th</sup> Street, Nelson Street, and the north end of 5<sup>th</sup> Street. Fieldwork identified numerous active rockfalls/rockslides as well as some debris slide areas that confirm the observations from the LiDAR and the air photo mapping (Figures 6, 7, and 8). Apparent landslide locations shown are based on poorly-vegetated slopes, swales and gullies seen on Google Earth that are also visible on LiDAR hillshade models (red arrows). Landmarks are as for Figure 14. (Image credit: CBJ 2013.)

The longest red arrow in Figure 15 shows the location of the debris flow gully within the G000 Park avalanche path. This gully appears to receive water and debris not only from the local slopes immediately above 5<sup>th</sup> Street, but also from surface drainage swale or gully that originates further upslope along the Mt. Roberts crest.

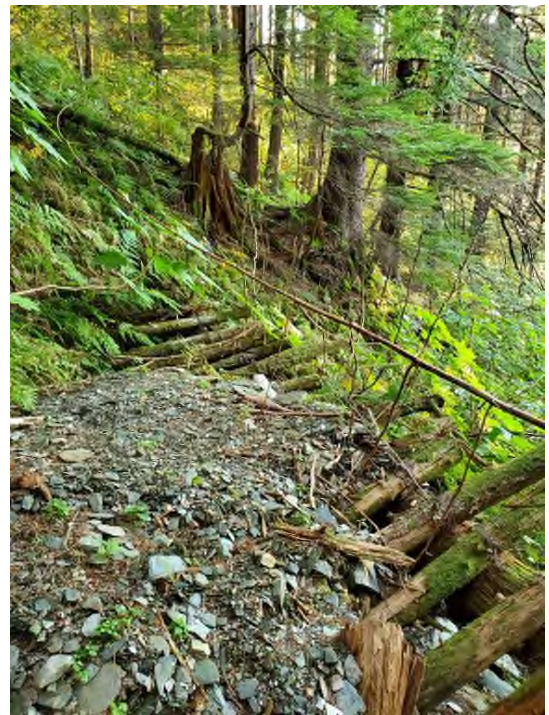
- 4. Question/Comment:** We agree that there is landslide activity in the [G000] Park gully, but properties have largely been protected by maintenance and instream mitigation structures and drains. The structures were installed by homeowners... with materials that were supplied by the City and Borough of Juneau. Some of these structures need to be repaired or replaced in order to continue proper drainage of this creek through the city-installed culvert that runs under the 725 5<sup>th</sup> Street home.

The area designated as *Severe* (red) has resulted in two debris flow events in the last 35 years... A debris flow incident occurred in the late 1980s, as a result of the drain above 725 5<sup>th</sup> Street becoming plugged because the home was vacant and there was no one to monitor/maintain the drain. In 2019, the gully creek undercut a bank, causing a flowerpot to fall and temporarily block the flow.

**Answer:** The review of the slopes in Question #3 provides some useful background in the overall slope processes that are happening in this area of Starr Hill. Specifically, between Park Street and Kennedy Street,

there is a debris flow gully that was not identified in any of the previous studies, except possibly as the “slump” reported in Swanston (1972), as discussed in Section 2.1.2.

Although the residents have reported only two debris flow events in the past 35 years, this does not mean that the hazard is not significant. Based on the field observations, this debris flow gully has a high potential to affect downslope residences. Clearly, residents were concerned enough about the debris flow hazard to build structures to control debris flows, but these structures are not by any means engineered structures (Figure 16). Routing a debris-flow creek under a house also seems fraught, considering that the drain upslope has become plugged in the past, and that the slightest misstep upslope can create further havoc. (Case in point, the creek undercutting the stream bank and a fallen flowerpot blocking the flow in 2019.) Furthermore, as shown in Figure 15, it appears that this debris flow gully could potentially receive significantly more debris and water than just from the local slopes. As it stands, due to these multiple sources of hazard, Tetra Tech considers that this debris flow gully has been correctly designated as *Severe*.



**Figure 16: Photos from the fieldwork in September 2019 showing one of the existing debris-flow mitigation structures that were built by homeowners living below the G000 Park debris flow path above Starr Hill. This structure has captured debris material from one or more mass-wasting events and measured about 8 feet wide by 7 feet long by 2 feet deep.**

Depending on the size of the next debris flow, the structure shown in Figure 16 could retain a little more debris or, instead, it could collapse, be overrun by debris, or even be completely scoured out by a larger debris flow that could originate from further up the gully. The same applies to the other structures documented by residents. Debris can incorporate both large and small woody debris, as seen in these photos and the photos supplied by residents of the mid and upper reaches of the gully. The upslope portion of this debris flow feature is bowl-shaped, indicating the potential for small debris slides from the side slopes to fail and entrain debris in the gully, of which there is a significant amount. The particle size of the material that can be moved by a debris flow is

also important – some cobble- and boulder-sized material is also visible in the residents' photos. Despite the mitigation attempts, debris slides/flows could result in enough volume to impact the houses below.

5. **Question/Comment:** We question the high risk [orange] designation adjacent to the gully, as the topography is relatively dry, stable, and does not seem to foster conditions for any landslide, debris flow, or erosion.

**Answer:** The answer to this question can be applied to both areas mapped as *High* hazard terrain (orange) beside the G000 Park gully: the open slopes located to the southwest and northeast of the debris flow gully. The bowl-shaped terrain located upslope of the corner of Nelson and 5<sup>th</sup> Streets is now mapped as *Severe*, so this discussion no longer applies to that terrain (see Question #2). The surficial geology is the same in both areas. The review of the slopes in Question #3 provides some useful background in the slope processes that are happening in this area of Starr Hill. Figure 15 shows clearly the very rough and disturbed terrain that has resulted from highly active slope processes, particularly on the northeast side of the G000 Park gully. Figures 10, 11, and 12 show the paths along which the most frequent landslides (debris slides/flows) tend to occur northeast of the gully, and Figure 12 also shows the paths southwest of the gully.

On the right-hand side of Figure 12, for the area southwest of the G000 Park gully, there are two red arrows high on the slope and a longer red arrow on the lower slope. The two upper red arrows on Figure 12 show the main areas of slope instability activity higher on the slope and, as seen on Figure 15, this instability is related to the rock bluffs/cliffs upslope, resulting in rockfall/rockslides. The geology mapping shows that debris slides can occur in this terrain too. The lower longer arrow indicates a transition zone where most of the rockslide or debris slide material continues downslope. Due to the open-slope environment, this slope is not as hazardous as the debris flow gullies on either side. However, the prominent toe of slope at the edge of the residential area clearly shows the edge of this terrain unit, and the considerable proportion of ground with sparse or no tree cover upslope is indicative of ongoing slope instabilities. This is why this slope section has a *High* hazard designation.

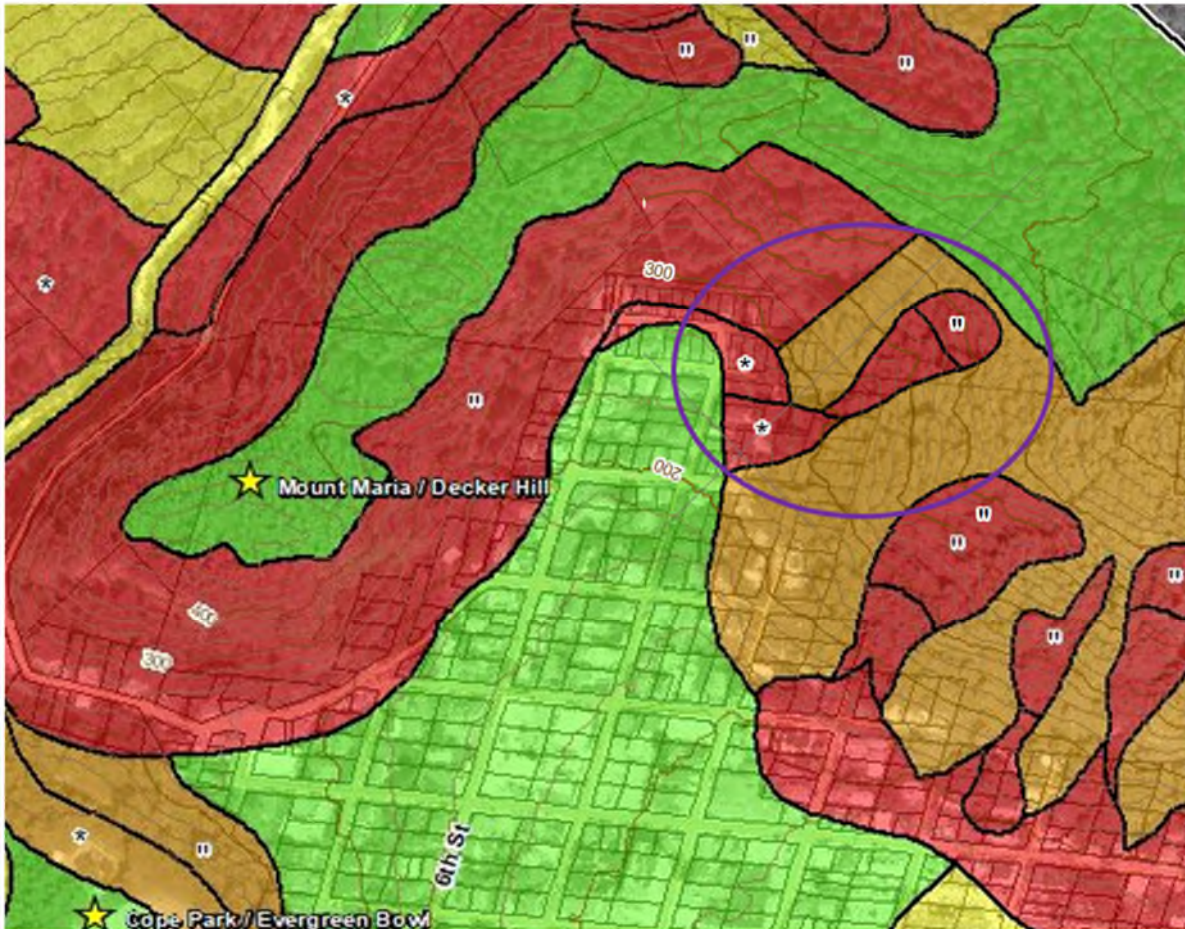


**Figure 17: Compare Tetra Tech’s photo from September 10, 2019 (Figure 17A) with residents’ photo from August 1, 2021 (Figure 17B) at the same location. Slope instabilities appear to be ongoing in the historical slide path locations.**

Note that local material volumes incorporated in debris slides can often be relatively small, but they are cumulative, and just as for debris flows, debris slides can include large and small woody debris. Eventually, there will be enough new or built-up debris combined with enough precipitation to bring the debris downslope to an elevation where it can cause damage. One recent example is the landslide that occurred on these slopes in November 2020 that was reported by a nearby resident in the online comments of the July 21, 2021, Neighborhood Meeting.

The primary distinction in hazards between slopes with debris slides and slopes with debris flow gullies is the mobility of the debris material. Debris flows are generally much more mobile than debris slides, and would be expected to run out further downslope, potentially affecting a much larger area, and thus warranting a *Severe* rating. However, every report from residents about landslides that have impacted their properties is important and will be taken into account when finalizing the landslide hazard designations in the Issued-for-Use report.

6. **Question/Comment:** The boundaries between the *Low*, *Moderate*, *High*, and *Severe* landslide hazard zones do not seem to match the land.



**Figure 18: Excerpt from the landslide hazard designation mapping. The purple outline shows an area where residents requested more information to understand the shapes of the mapping units and the reasons supporting the hazard designations.**

**Answer:** The boundaries between the different landslide hazard designation zones are closely related to the surficial geology mapping terrain unit boundaries (Figure 3A). The reason for this relationship is that the different soil and rock features have a large influence on how the slopes behave. For shapes that do not seem to make sense, it is helpful to look at the features beside that odd shape. Usually, it will be a terrain feature whose characteristics will govern the shape of the boundary between the two units, like a bedrock outcrop, or a terrain unit that overlaps a previous unit. Sometimes, the odd-looking boundary is only because there is a corner or curve in the slope, so that debris from one side falls down in one direction, and material from the other side falls down in a different direction. These two debris areas might then meet at the bottom of the slope, like the southeast corner of Starr Hill, where debris can fall or slide from above Nelson Street, and it can also fall or slide or flow from above 5<sup>th</sup> Street. Since this part of the subdivision is essentially the shape of a bowl, the mapping of the unit boundaries can also reflect slope contours and fall-lines, as well as the surficial geology unit boundaries. See also Technical Memo #2, Question #3, for more explanation on how apparently odd-shaped boundaries are determined.

Specific to this area of Starr Hill, it is now known that a resident has identified a recent landslide event in a *High* hazard zone (see Question #6 above), which also happened to be within one of the apparently odd-shaped hazard units (Figure 18, purple outline). This area has now been updated, resulted in a shifting of the hazard boundary above the corner of Nelson Street and 5<sup>th</sup> Street. Note that the *Severe* hazard zone mapped at the toe of slope is due to this area being a deposition zone for slide debris.

If other property owners have experienced landslide events – rockfalls, rockslides, debris slides, debris flows, and so on – at their properties, not just in Starr Hill, but anywhere in the downtown Juneau study area, this is the time to report those landslides, to help finetune the mapping. When reporting landslides, please report if there was damage and, if so, what was damaged and to what extent, for example, structures or landscaping. If quantities of debris removed or cleaned up are known, please report approximate quantities also.

7. **Question/Comment:** The maps raise significant questions as to how areas were given certain designations. Houses that have had tree slides damaging the structure have been included in lower hazard zones than those below with no tree slide history. How are some areas adjacent to severe hazard zones rated as low hazard zones without a transition area? What site specific analysis was done in each area, such as Starr Hill?

**Answer:** As noted above in the answers to Questions #3, #4, and #6, information about landslide events is important to improve the accuracy of the mapping, especially where these events may not be visible on the air photos or the LiDAR, or events that are not part of the historical record. Where such information has been provided, it is used to confirm or update the mapping, as applicable.

Landslide hazards are generally not downgraded in a downslope direction. If there is a history of slide activity, or if it is a runout zone (i.e., a deposition zone), an area is considered to pose a severe hazard. Note that not all landslide events begin at the top of the mapped feature, nor do they necessarily extend to the bottom of the feature, which can be seen clearly in the historical air photo record and LiDAR data analysis, in both the slope movement features (Figures 1.4a to 1.4c) and the gully erosion features (Figures 1.5a to 1.5c). See also Technical Memo #2, Question #2.

For general information on how the landslide designations are determined, Technical Memo #2 provides a good summary. In general, the landslide hazard mapping shapes follow the shapes of the types of ground that they represent, and this is true for Starr Hill also. Using the air photos and other imagery, Tetra Tech targeted the areas that specifically needed to be visited in the field. A foot traverse was done around the slopes of the Starr Hill subdivision to confirm, correct, or add to the information collected from the imagery. A greater concentration of field observations were made on slopes above residential areas.

8. **Question/Comment:** How are severe hazard zones with a 300 foot run from the ridge above a residence compare to those with 3,000 foot runs? Are these actually comparable situations?

**Answer:** When comparing debris slides, the length of the mountain slope does not necessarily determine the length of the debris slide. For example, comparing the sizes of debris slides mapped above Nelson and 5<sup>th</sup> Streets to the sizes of debris slides further southeast on the main slope of Mt. Roberts, most of them are very similar. Where the debris slides do tend to be larger (or longer) on the larger slopes, they are usually associated with gullies that have steeper sideslopes, or with large open avalanche slopes (more typically on Mt. Juneau, but also south of Snowslide Creek), and usually on high-elevation terrain – see Figures 1.4a, 1.4b, and 1.4c in Tetra Tech's report. This can be important where high-elevation debris slides end up in long gullies where debris flows are active, and the size of the initiation zones reflect that.

When comparing gullies, long gullies do not always mean that a debris flow event will extend along the entire length of the gully every time it flows – notice all the shorter arrows of different colours on Figures 1.5a, 1.5b,

and 1.5c in Tetra Tech's report. However, there are some major gullies which do experience debris flows, at least some of the time over a significant proportion, if not all, of the gully length. The degree of the hazard is shown not only by the hazard designation – always *Severe* for debris flows, but also by the size of the cone/fan that receives debris from the gully – the runout or deposition zone. For example, compare the size of the cone/fan at Bathe Creek to the size of the cone/fan below 3<sup>rd</sup> and Harris Streets, or between Kennedy and Park Streets above 5<sup>th</sup> Street. The size of the receiving area for debris at the toe of slope correlates roughly to the upslope terrain providing debris to the gullies, within or along the gullies.

In general, an area is given a hazard designation of *Severe* if:

- A cone or fan of colluvium is present at the base of a slope, no matter how old it is, because the hazard still exists (Howes and Kenk 1997); and/or
- Evidence of slope instability (exhibited on air photos as a lack of vegetation in a formerly vegetated area with an obvious downslope movement component; incident reports; and/or field observations) is identified within the same feature in more than one air photo/LiDAR year and/or field investigation year.

These criteria are met for numerous landslide features around the Starr Hill subdivision. Technical Memo #2 provides more information on how landslides are evaluated.

9. **Question/Comment:** I don't understand why my property is now in a *Low* hazard zone. My property never used to be in any zone at all, and now I don't know if my property is at risk for landslides. I would also like to know more about the geology and hazards that are present directly above my property.

**Answer:** In the current adopted hazard mapping system, two hazard zone designations were specifically mapped: *Moderate* Hazard Zone (or Special Engineering Zone in some of the references) and *Severe* Hazard Zone (or *High* Hazard Zone in some of the references). Anything outside those two mapped zones was not specifically considered in the old mapping. Including the new hazard designation of *Low* for both avalanche and landslide hazards will make the mapping system consistent with numerous internationally accepted hazard mapping systems. In the case of avalanche hazards, everything not mapped as *Moderate* or *Severe* is considered *Low*. In the case of landslide hazards, everything not mapped as *Moderate*, *High*, or *Severe* is considered *Low*.

This does not mean that the hazard has changed for properties that are now designated as being in a *Low* hazard zone. It just means that it has been given a name that recognizes that a hazard is never "zero," but the hazard is low enough that owners of properties within the *Low* hazard zone should not have to do anything extra to protect their properties from avalanches or landslides, except for being attentive, i.e., observing and recording anything unusual at or around their properties, such as ground settlement, cracking etc. See the definitions for Avalanche Hazard Designation and Landslide Hazard Designation in the glossary of the Tetra Tech report. Note that the estimated event probabilities for landslide hazard designations have been updated to a format similar to the return periods reported in the avalanche study. See also the discussions in Technical Memo #2, Question #1, and Technical Memo #4 (both in Appendix C of the main report; Tetra Tech 2022b, 2022d).

The only caveat to this answer is that if there was a landslide (like a rockfall, rockslide, debris slide, or debris flow) that resulted in debris ending up at, beside, or very close to, your property; or a house upslope of your property was damaged due to a landslide and now that house is gone, the boundary between hazard zones might need to be adjusted. For debris that is cleaned up after a landslide happens, or for former houses that did not appear on any of the air photos, the mapping cannot always detect where landslides might have occurred. That means the mapping also needs to be supported by good historical records, including property owner reporting, if applicable and available.

10. **Question/Comment:** My house is over 90 (or 100) years old and still standing. How can I be in a *Severe* hazard zone? I don't recall anything happening to my house in the 25 (or 50) years I've lived here, and the neighbours don't remember anything either.

**Answer:** See the bottom part of the answer to Question #6 above, about how an area is designated as being in a *Severe* hazard zone. Sometimes the hazard is not related to what is happening right around your house, but what is happening higher on the slope or around your neighbour's house. That is especially true for hazards related to debris flows, because where the debris will end up is not always predictable. See also Technical Memo #2, Question #8 for more information. Also, residents might not always know what happened to their lot or house before they moved there.

11. **Question/Comment:** I feel the historical timeline and perspective on how the mapped risk areas have changed is not given enough consideration. How can we get more information on how changing conditions, geology, and climate affect slope stability? Some of the changes are due to human-altered landscapes, like clearcut logging or rock cuts. Has the city reached out to geotechnical experts on rock type behavior, slope angles, vegetation, and historical and future angle of repose? Often those questions can only be answered by drilling and core analysis.

**Answer:** Tetra Tech's project team of engineers and geoscientists provided expertise for this project. Tetra Tech's report provides a full description of the procedures used to evaluate the slopes, such as mapping of surficial geology and confirmation of surface materials during the fieldwork, including areas mapped as anthropogenic (human-modified) terrain. Information on changing vegetation (for example, as a result of landslides), slope angles, and surficial geology can all be obtained by means of desktop study terrain analysis (which included air photo and LiDAR data analysis), mapping, and confirmatory fieldwork. Rock types and characteristics were recorded by Tetra Tech's highly qualified and experienced engineering geologist/geotechnical engineer where bedrock was exposed at ground surface. The evaluation of engineered rock cuts or other engineered slope mitigations like retaining walls was not in the project scope. Geotechnical drilling was also not in the project scope, nor was an evaluation of climate change. See Tetra Tech's report, as well as Technical Memos #1 and #2 for additional information on the methods of evaluation, as well as the limitations of the work. Question #1 above addresses clearcutting.

12. **Question/Comment:** Although the old Mt. Roberts trailhead at the top of 6<sup>th</sup> Street was supposedly abandoned years ago, it continues to receive regular, year-round (and likely daily) use by locals and visitors alike. This use by hikers and runners is likely destabilizing the hillside above the Nelson Street homes and worsening the landslide conditions, especially because the trail is no longer maintained, and hikers have made their own shortcuts. The current signage and availability of stair access does more to invite users than it does to discourage them. The CBJ should consider removing the stairs and placing signage that strongly discourages users by explaining that foot traffic is causing erosion, destabilizing the hillside, and threatening the homes below. Other strategies could include educational outreach to local hiking clubs and local guides, and updating local trail maps.

**Answer:** Figures 9 and 17 show some typical slope sections above Nelson Street. Eliminating access to sensitive slopes that also pose a safety hazard to trail users is an important strategy used in many jurisdictions. Even after the stairs have been removed, physically blocking access with sections of fencing might also be necessary to deter ambitious hikers. Interpretive signage can also help, especially if there are other elements of value that would be preserved by deterring foot traffic.

The trail should not be simply abandoned and ignored. Control of surface water drainage may be very important on the deactivated trail section, especially where there are switchbacks with no intermediate water management provisions along the trail. This is because trails (especially in-sloped ones) tend to concentrate surface water



drainage over long sections of trail, until accumulated water from numerous small streams and swales all run downslope at the end of a switchback. Shortcutting is particularly common on trails with switchbacks and, in addition to erosion resulting from foot traffic, shortcuts can result in additional slope sections with concentrated surface water drainage. At best, concentrated water can result in soil erosion; at worst, it can result in slope failures. Therefore, when deactivating the trail, the original drainage paths across the trail should be restored. These same considerations apply to active trail sections – good control of surface water drainage will improve slope stability.

13. **Question/Comment:** The study (or at least the new regulations) should address how structures factor into landslide hazards. The new zones were drawn agnostic of human-made structures, like buildings, above us. But realistically the structures exist and will mitigate landslides. That means many, if not hundreds of homes not actually at risk of a landslide will be classified as if they were, which benefits no one.

**Answer:** The premise of this comment is that upslope structures will always protect the structures downslope. However, this is not always true. Sometimes the upslope structures are simply incorporated into the debris, adding more mass to damage or destroy the downslope structures. A classic example of this kind of event is the January 2, 1920 landslide that occurred between Decker Way and Bulger Way, destroying 16 buildings from Gastineau Avenue to Front Street (now South Franklin Street). That landslide resulted in numerous buildings sliding downslope with the debris, overrunning other structures and destroying them too. See Question #14 and Technical Memos #5 and #7 (in Appendix C of the main report; Tetra Tech 2022e, 2022g) for more information about the landslide.

14. **Question/Comment:** Why are past landslides used as indicative, without accounting for the fact that some were on deforested slopes undergoing blasting and water discharge from mining?

**Answer:** A detailed review of mining practices including blasting and water discharges was not within the project scope. However, terrain where the ground surface was drastically modified by human activities such as mining, cutting into slopes or placing fill, where visible on the air photos, was mapped as “anthropogenic” terrain. These modifications of geological material have been mapped along a significant length of the map area along the toe of Mt. Roberts, as seen in the cross-hatched areas shown on Figures 1.3b and 1.3c in the Tetra Tech report. The closest anthropogenic terrain to the Starr Hill subdivision is located southeast of 4<sup>th</sup> Street and northeast of Gold Street, just around the corner onto the main slope of Mt. Roberts, where a cone/fan-shaped area is the runout zone for debris from the upslope debris flow gully. Most of the modifications of this terrain appear to be related to residential development.

It is understood that the Alaska Juneau Gold Mining Company (AJGMC) had its mill on the slope of Mt. Roberts near the southeast end of the historical downtown area. The AJGMC mill began operating in 1917, with the mine operating 24 hours per day and 363 days per year by 1930. The mine was closed in 1944.

Drilling and blasting would have been part of the operations associated with the Alaska-Juneau mine, along with the Ebner and Perseverance workings, accessed from Last Chance Basin along Gold Creek or from the Sheep Creek Tunnel. There was also a tunnel upslope of the former office above Gastineau Avenue, with the first portal completed in 1913 near the north end of the tramway, and the second portal completed in 1916 about 500 feet further to the southeast, between the north portal and the AJGMC Mill. Blasting would not have taken place at the mill, although crushing of the ore might well have resulted in some vibrations during operations. The Starr Hill subdivision was about 0.5 miles and 0.7 miles west of the entrances of the Ebner and AJGMC adits, respectively, roughly 2.0 miles to 2.5 miles west of the top ends of all the adits at Silverbow Basin, 0.3 miles to 0.5 miles northwest of the AJGMC tunnel, and about 0.7 miles northwest of the AJGMC Mill, based on the 1914 topographic map of Juneau. No blasting-related or vibration-related slope instabilities were

mentioned in any news reports so far reviewed for the landslides at Mt. Roberts specifically, nor for Juneau in general.

The AJGMC flume was reported to have overflowed at the time of the January 2, 1920 landslide. Since there was also melting snow and nearly 2 inches of heavy rain in 24 hours (Swanston 1972), the overflow of the flume might have contributed to that debris slide but was likely not the only cause. In a photo from the Gastineau Channel, at least two streams of concentrated water were running downslope, one within the landslide area and one to the north (Alaska State Library, Photo ASL-P87-1223).

Surficial geology mapping by Miller (1975) shows that another landslide occurred on October 1, 1952 at the same location as the 1920 landslide, again after nearly 2 inches of rain in 24 hours. Since the mine had closed eight years prior, water from the flume should not have been a factor in the 1952 landslide, and no mine-related complaints have been found thus far in relation to the 1952 slope failure. That landslide resulted in the closure of South Franklin Street by the old Columbia Lumber Co. kiln. The exact location of the kiln is not known, but it could have been either between Decker Way and Bulger Way, or in the vicinity of 475 South Franklin Street, and likely belonged to the new plywood plant of its subsidiary Columbia Plywood Co. Two structures were also destroyed in 1952, due to landslides at 261 Gastineau Avenue and 475 South Franklin Street, located further southeast along Mt. Roberts (Swanston 1972). Notably, a small house was built at 261 Gastineau Avenue sometime after 1920 – at the same location where two houses had been destroyed in the 1920 landslide. The 475 South Franklin Street landslide would have been located within, or very close to, the path of the major landslides that occurred on November 22, 1936.

In the November 22, 1936, landslide, a tension crack was noted at a slope failure below the flume, and this location also appears to have been the approximate initiation zone for that event, based on the appearance of the vegetation on the 1971 map of Juneau. Water from the flume was not directly implicated in that event, however. If there was a leak, it might or might not have been significant compared to the nearly 4 inches of rain that fell in 24 hours (Swanston 1972). In any case, the initiation zone for the overall debris flow feature is nearly at the top of the ridge, indicating that no leaky flume would be necessary to trigger another landslide. No reports so far reviewed have implicated flume leakage in any of the landslide areas on the slopes above Starr Hill.

Upslope of almost all this human-modified terrain, there are debris flow gullies, originating in natural terrain. Those natural debris flow gullies are the source of the material that runs out onto the cones/fans along the toe of slope. Even after the removal of all mining-related structures and activities, those upslope debris flow gullies remain as the most significant sources of landslide hazards on this slope.

### 3.0 LIMITATIONS OF REPORT

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## GEOTECHNICAL

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The Client, and any Authorized Party, acknowledges that the Professional Document is based on limited data and that the conclusions, opinions, and recommendations contained in the Professional Document are the result of the application of professional judgment to such limited data.

The Professional Document is not applicable to any other sites, nor should it be relied upon for types of development other than those to which it refers. Any variation from the site conditions present, or variation in assumed conditions which might form the basis of design or recommendations as outlined in this report, at or on the development proposed as of the date of the Professional Document requires a supplementary investigation and assessment.

TETRA TECH is neither qualified to, nor is it making, any recommendations with respect to the purchase, sale, investment or development of the property, the decisions on which are the sole responsibility of the Client.

## 1.7 ENVIRONMENTAL AND REGULATORY ISSUES

Unless stipulated in the report, TETRA TECH has not been retained to investigate, address or consider and has not investigated, addressed or considered any environmental or regulatory issues associated with development on the subject site.

## 1.8 NATURE AND EXACTNESS OF SOIL AND ROCK DESCRIPTIONS

Classification and identification of soils and rocks are based upon commonly accepted systems and methods employed in professional geotechnical practice. This report contains descriptions of the systems and methods used. Where deviations from the system or method prevail, they are specifically mentioned.

Classification and identification of geological units are judgmental in nature as to both type and condition. TETRA TECH does not warrant conditions represented herein as exact, but infers accuracy only to the extent that is common in practice.

Where subsurface conditions encountered during development are different from those described in this report, qualified geotechnical personnel should revisit the site and review recommendations in light of the actual conditions encountered.

## 1.9 LOGS OF TESTHOLES

The testhole logs are a compilation of conditions and classification of soils and rocks as obtained from field observations and laboratory testing of selected samples. Soil and rock zones have been interpreted. Change from one geological zone to the other, indicated on the logs as a distinct line, can be, in fact, transitional. The extent of transition is interpretive. Any circumstance which requires precise definition of soil or rock zone transition elevations may require further investigation and review.

## 1.10 STRATIGRAPHIC AND GEOLOGICAL INFORMATION

The stratigraphic and geological information indicated on drawings contained in this report are inferred from logs of test holes and/or soil/rock exposures. Stratigraphy is known only at the locations of the test hole or exposure. Actual geology and stratigraphy between test holes and/or exposures may vary from that shown on these drawings. Natural variations in geological conditions are inherent and are a function of the historic environment. TETRA TECH does not represent the conditions illustrated as exact but recognizes that variations will exist. Where knowledge of more precise locations of geological units is necessary, additional investigation and review may be necessary.

## 1.11 PROTECTION OF EXPOSED GROUND

Excavation and construction operations expose geological materials to climatic elements (freeze/thaw, wet/dry) and/or mechanical disturbance which can cause severe deterioration. Unless otherwise specifically indicated in this report, the walls and floors of excavations must be protected from the elements, particularly moisture, desiccation, frost action and construction traffic.

## 1.12 SUPPORT OF ADJACENT GROUND AND STRUCTURES

Unless otherwise specifically advised, support of ground and structures adjacent to the anticipated construction and preservation of adjacent ground and structures from the adverse impact of construction activity is required.

## 1.13 INFLUENCE OF CONSTRUCTION ACTIVITY

There is a direct correlation between construction activity and structural performance of adjacent buildings and other installations. The influence of all anticipated construction activities should be considered by the contractor, owner, architect and prime engineer in consultation with a geotechnical engineer when the final design and construction techniques are known.

## 1.14 OBSERVATIONS DURING CONSTRUCTION

Because of the nature of geological deposits, the judgmental nature of geotechnical engineering, as well as the potential of adverse circumstances arising from construction activity, observations during site preparation, excavation and construction should be carried out by a geotechnical engineer. These observations may then serve as the basis for confirmation and/or alteration of geotechnical recommendations or design guidelines presented herein.

## 1.15 DRAINAGE SYSTEMS

Where temporary or permanent drainage systems are installed within or around a structure, the systems which will be installed must protect the structure from loss of ground due to internal erosion and must be designed so as to assure continued performance of the drains. Specific design detail of such systems should be developed or reviewed by the geotechnical engineer. Unless otherwise specified, it is a condition of this report that effective temporary and permanent drainage systems are required and that they must be considered in relation to project purpose and function.

## 1.16 BEARING CAPACITY

Design bearing capacities, loads and allowable stresses quoted in this report relate to a specific soil or rock type and condition. Construction activity and environmental circumstances can materially change the condition of soil or rock. The elevation at which a soil or rock type occurs is variable. It is a requirement of this report that structural elements be founded in and/or upon geological materials of the type and in the condition assumed. Sufficient observations should be made by qualified geotechnical personnel during construction to assure that the soil and/or rock conditions assumed in this report in fact exist at the site.

## 1.17 SAMPLES

TETRA TECH will retain all soil and rock samples for 30 days after this report is issued. Further storage or transfer of samples can be made at the Client's expense upon written request, otherwise samples will be discarded.



**To:** Teri Camery (CBJ) **Date:** April 27, 2022  
**c:** Scott Ciambor (CBJ) **Memo No.:** 4  
**From:** Rita Kors-Olthof, Alan Jones, Vladislav Roujanski **File:** 704-ENG.EARC03168-02A  
**Subject:** Guide to Avalanche and Landslide Hazard Designations  
Downtown Juneau Landslide and Avalanche Hazard Assessment

## 1.0 INTRODUCTION

Tetra Tech Canada Inc. (Tetra Tech) has prepared an Issued-for-Review (3<sup>rd</sup> Draft) Report, Downtown Juneau Landslide and Avalanche Assessment for the City and Borough of Juneau (CBJ), dated May 28, 2021 (Tetra Tech 2021); and participated in three Landslide and Avalanche Hazard Public Meetings that took place on July 21, August 10, and September 20, 2021.

Tetra Tech has provided a series of technical memos to respond to comments and questions that arose from the from the report and the public meetings. All the completed memos will be appended to the Final Draft Report.

This Technical Memo #4 provides a “Guide to Avalanche and Landslide Hazard Designations.” More in-depth explanations for landslides are also provided to respond to questions and concerns from the public, and in recognition of the larger number of variables and challenges in predicting behavior for landslides compared to avalanches. The primary objective of this memo is to help Juneau residents and CBJ better understand the meanings of the avalanche and landslide hazard designations. The secondary objective is to provide some additional background to help understand the limitations of those hazard designations. A quick-reference table for the contents of this memo is presented in Table 1.

**Table 1: Quick-Reference Table for the Contents of the Guide**

Section Number	Section Heading	Page Number
1.0	Introduction	1
2.0	Avalanche Hazard Designations and Descriptions	2-14
3.0	Landslide Hazard Designations and Descriptions	14-29
4.0	Hazard from Above or Hazard from Below	29-30
5.0	Limitations of a Hazards-Only Assessment	30
6.0	Requests for Additional Information	31-32



## 2.0 AVALANCHE HAZARD DESIGNATIONS AND DESCRIPTIONS

### 2.1 General

This section will provide information on avalanches, including:

- The definition of an avalanche;
- Definitions of the avalanche hazard designations;
- Excerpts from the mapping to show examples of each designation;
- Photos with examples of the terrain in each of the hazard designations; and
- An explanation of the limitations of a hazards-only assessment.

### 2.2 What is an Avalanche?

An avalanche means a snow avalanche, unless otherwise specified, and is it usually just called an “avalanche.” A snow avalanche is a volume of snow moved by gravity, that is visibly moving downslope. Snow avalanches can contain rock, broken trees, soil, ice, or other material in addition to snow (after CAA 2016).

### 2.3 How are Avalanche Hazards Designated?

Avalanche hazard designations are based on review of snow climate data, previous reports and studies, historic avalanche occurrence records, magnitude-frequency analyses, air photos, satellite imagery, LiDAR data, field investigation, meetings and data provided by local experts, and dynamic and statistical avalanche modelling.

The Downtown Juneau Study Area was divided into areas with *Low*, *Moderate*, and *Severe* avalanche hazard designations, according to the results of the analysis for each of the avalanche areas. The *Low*, *Moderate*, and *Severe* zones are often called White, Blue, and Red hazard zones in other jurisdictions (as they are in several of the references used for this project), and those are the colors assigned to them in the mapping shown on Figures 2.3a, 2.3b, and 2.4a through 2.4j. This system is based on a combination of magnitude (impact pressure) and frequency, with CBJ designations consistent with those used in Europe and Canada. Avalanche paths were mapped to delineate a 300-year hazard boundary for destructive flow (dense and/or powder avalanches). Table 2 shows the avalanche hazard designation system. This table is the same as Table 2.3 in the main report.

**Table 2: Avalanche Hazard Designation System**

Hazard Designation	Symbol	Hazard Attribute Description
<i>Low</i>	L	<ul style="list-style-type: none"> <li>▪ Return period greater than 300 years; OR</li> <li>▪ Impact pressures less than 20 lbs/ft<sup>2</sup> (1 kPa) with a return period greater than 30 years.</li> </ul>
<i>Moderate</i>	M	<ul style="list-style-type: none"> <li>▪ Return period between 30 and 300 years; AND</li> <li>▪ Impact pressure less than 600 lbs/ft<sup>2</sup> (30 kPa).</li> </ul>
<i>Severe</i>	S	<ul style="list-style-type: none"> <li>▪ Return period less than 30 years; AND/OR</li> <li>▪ Impact pressure greater than or equal to 600 lbs/ft<sup>2</sup> (30 kPa).</li> </ul>

There are some important differences between the new hazard designation mapping and the adopted 1987 hazard designation mapping:

- The 1987 mapping and the current mapping have slightly different boundaries due to different project areas. These differences resulted in some areas being flagged as concerns, when the differences were in fact due to new areas being mapped that had not been mapped before (additional Study Area northwest and southeast, and to reach the top-of-slope or ridge crest), or areas being omitted in the new mapping because they were beyond the top-of-slope boundary line of the new Study Area. Different modelling methods also led to differences in estimated runouts, which were particularly prominent where they extended into Gastineau Channel.
- The 1987 mapping combined avalanche and landslide hazard designations into one map. As it turns out, avalanche hazard designations and landslide hazard designations tend to be very different, and they should not be grouped together into the same maps. The new maps show landslide and avalanche hazard designations on different maps, so that they can be managed independent of each other.
- The 1987 mapping follows property lines, resulting in numerous right-angle corners in the hazard boundaries. Avalanches do *not* respect property lines, instead running right over them, and forming boundaries that relate only to the conditions that create avalanches, such as slope gradients, topography, snow conditions, wind, winter storms, rain-on-snow events, and rapid spring melt conditions, among other factors. The new avalanche hazard mapping does not follow property boundaries, but rather reflects observed and modelled avalanche behavior combined with historical observations.
- Structures located in avalanche paths do not provide protection, and thus the avalanche hazard lines are “agnostic” to the structures.
- Due to these limitations, arbitrary hazard boundaries that follow property lines should be removed as not reflecting the true threat to the public safety, i.e., hazard designations based on property lines do **not** adequately describe the hazards.

The level of assessment prepared for this project is suitable for determining whether land areas could be affected by avalanches. A more detailed site-specific investigation and evaluation would be required to determine appropriate mitigations for specific properties.

## 2.4 Avalanche Hazard Designation - Low

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An avalanche hazard designation of *Low* is used for avalanches that have a return period of more than 300 years, OR avalanches with impact pressures of less than 20 lbs/ft<sup>2</sup> (1 kPa) with a return period of more than 30 years. Allowing a low impact pressure means that non-destructive powder avalanches can enter *Low* hazard areas, which is common in Juneau (e.g., Snowslide Creek path on Thane Road) and should be considered acceptable. For reference, 20 lb/ft<sup>2</sup> or 1 kPa could be capable of breaking windows or snapping tree branches but, for the most part, is not considered harmful to people or structures, which is why it is used as part of the *Low* hazard designation.

An estimate of the return period of 300 years or 30 years for an avalanche is the same as calling it a 1 in 300-year event or a 1 in 30-year event. Note that the return period of an avalanche does *not* mean that an event of a specified size or severity will return *every* X number of years. It just means that, on average, one could expect an avalanche of about that size or severity about that often, but the actual return period could be shorter or longer. For a 30-year return period, for example, the typical range in the return period is 20 years to 50 years, as shown in Table 2.1 in the main report. However, if one observes consistently longer or shorter return intervals than the average, the avalanche experts might eventually decide to assign a different return period to that size of avalanche. A change in return period could occur due to a number of reasons, including climate change, changes in forest cover, or terrain modification by natural (e.g., landslides) or human-induced (e.g., mining) causes.

On the avalanche hazard designation mapping, a *Low* avalanche hazard zone is considered to be the same as the White zone, which means there is no extra color added to the map. The *Low* avalanche hazard zones are located anywhere that is not colored blue or red on the accompanying avalanche hazard maps.

Residents who suddenly find their property assigned a *Low* hazard designation, after never being in a named zone before, might wonder what that means. Including a *Low* hazard designation makes the mapping system consistent with numerous internationally-accepted hazard mapping systems. This does not mean that the hazard has changed for properties that are now designated as being in a *Low* hazard zone. It just means that it has been given a name that recognizes that a hazard is never “zero,” but the hazard is low enough that owners of properties within the *Low* hazard zone generally should not have to do anything extra to protect their properties from avalanches, except for being attentive, i.e., observing and recording anything unusual at or around their properties, such as avalanche debris coming closer to the house than usual etc. The caveat to that logic could be if something changes around your property, like a structure being removed, or if the debris from an avalanche wasn’t recorded before it was removed, making it difficult to detect where it occurred. See Question #9 on Tech Memo #3 for more information.

One example of terrain with an avalanche hazard designation of *Low* is most of the Starr Hill subdivision, as shown in Figure 1. Figure 2 shows a view of Starr Hill from the helicopter.



Figure 1: Excerpt from Figures 2.4c and 2.4h in the main report, showing the northeast end of the Starr Hill subdivision. Almost all of the lots are mapped with an avalanche hazard designation of *Low* (i.e., not colored as red or blue), with the only encroachment being the G000 (Park) avalanche path on the right (marked *Severe*, with *Moderate* terrain below). All the existing houses are currently located in areas designated as *Low*.



**Figure 2: Looking southeast at the Starr Hill subdivision. Nelson Street is near the top left of the photo, East Street is near the photo center, Gold St. is near the photo right edge. 6<sup>th</sup> Street is in the foreground left, and the next road to the southeast is 5<sup>th</sup> Street.**

## 2.5 Avalanche Hazard Designation - *Moderate*

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An avalanche hazard designation of *Moderate* is used for areas that have a return period between 30 and 300 years AND have an impact pressure of less than 600 lbs/ft<sup>2</sup> (30 kPa). To compare, Table 2.2 in the main report describes some typical avalanche sizes, and what an avalanche of a specified size might be expected to do.

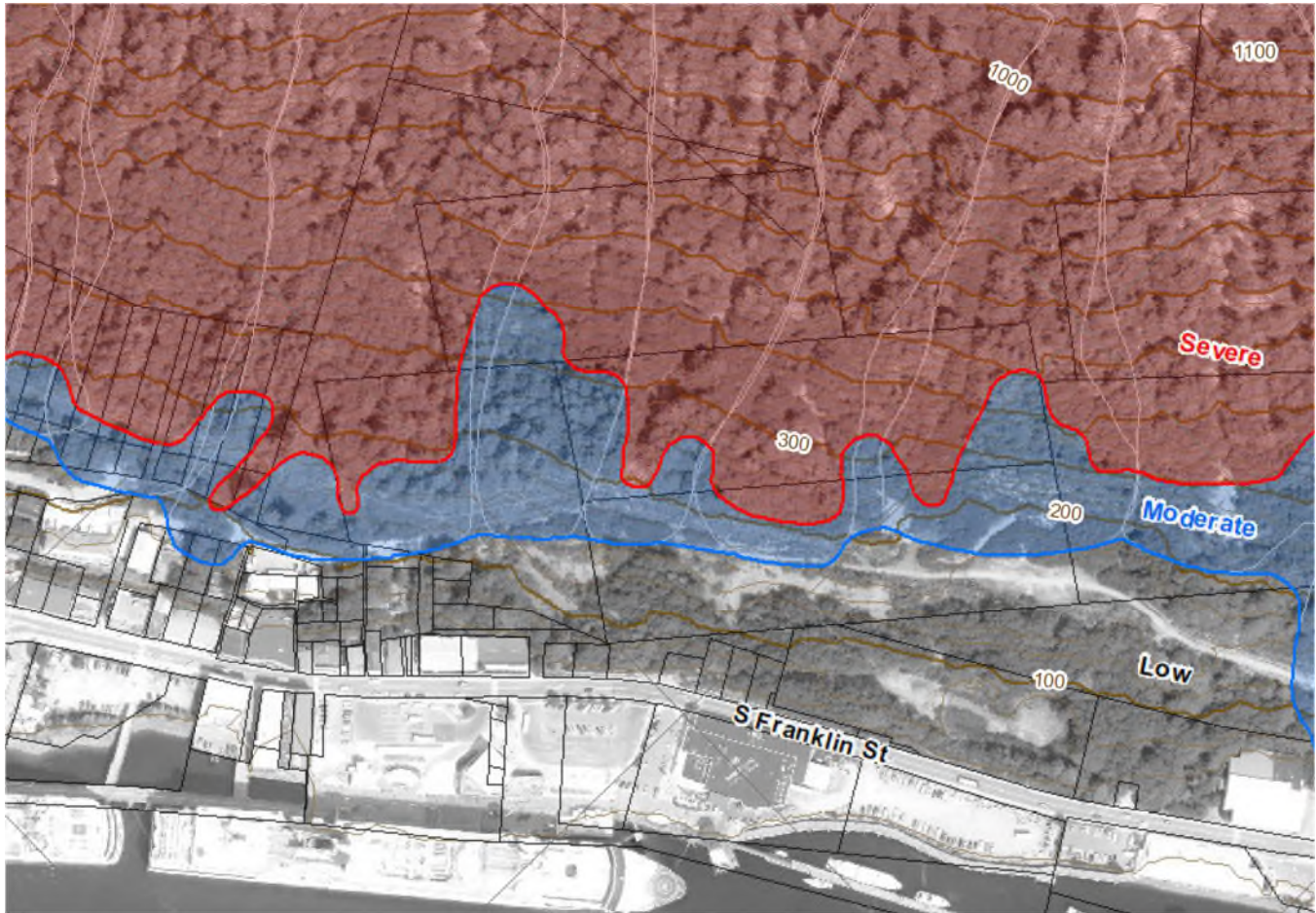
For example, a Size D2 avalanche that could produce a typical impact pressure on the order of 200 lbs/ft<sup>2</sup> (10 kPa) could bury, injure, or kill a person (e.g., a person outside of a house in their back yard). On the other hand, a Size D3 avalanche [typical impact pressure on the order of 2,000 lbs/ft<sup>2</sup> (100 kPa)] could bury and destroy a car, damage a truck, destroy a wood frame house, or break a few trees. An impact pressure of 600 lbs/ft<sup>2</sup> is typically used as a threshold between the *Severe* and *Moderate* hazard designations because it is close to the threshold that destructive avalanches (i.e., Size D3 or larger) typically can destroy wood-frame structures and thus kill people within them, whereas below this threshold they typically just damage rather than destroy the structures (and thus

are less likely to kill the occupants). It's important to point out that avalanches with impact pressures less than 600 lbs/ft<sup>2</sup> (30 kPa) can still cause considerable damage to residences and kill people, but would be expected to do so less frequently (or, alternatively, less severely) than in areas designated as red (*Severe*) hazard zones. Table 3 provides a summary of impact pressures associated with various types or extent of damage.

**Table 3: Impact Pressures Associated with Damage (modified from CAA 2018)**

Potential Damage	Impact Pressure	
	lbs/ft <sup>2</sup>	kPa
Break windows	21	1
Push in doors, damage walls, roofs	62-125	3-6
Severely damage wood frame	209	10
Destroy wood frame structures, break trees	418-626	20-30
Destroy mature forests	1,044-2,090	50-100
Uproot mature spruce	2,090	100
Move large boulders	6,262	300
Move reinforced concrete structures	20,900	1,000

Avalanche areas mapped with a hazard designation of *Moderate* are shown in blue on the mapping. Typically, the *Moderate* zone on the larger mountain slopes forms a fringe downslope and alongside the main avalanche paths (mapped in red) that is less likely to experience an avalanche, and if an avalanche does reach *Moderate* terrain, the impact pressures are expected to be lower, and are impacted less frequently. Figure 3 is an example of this type of avalanche terrain adjacent to the southern section of Gastineau Avenue and upslope of South Franklin Street.



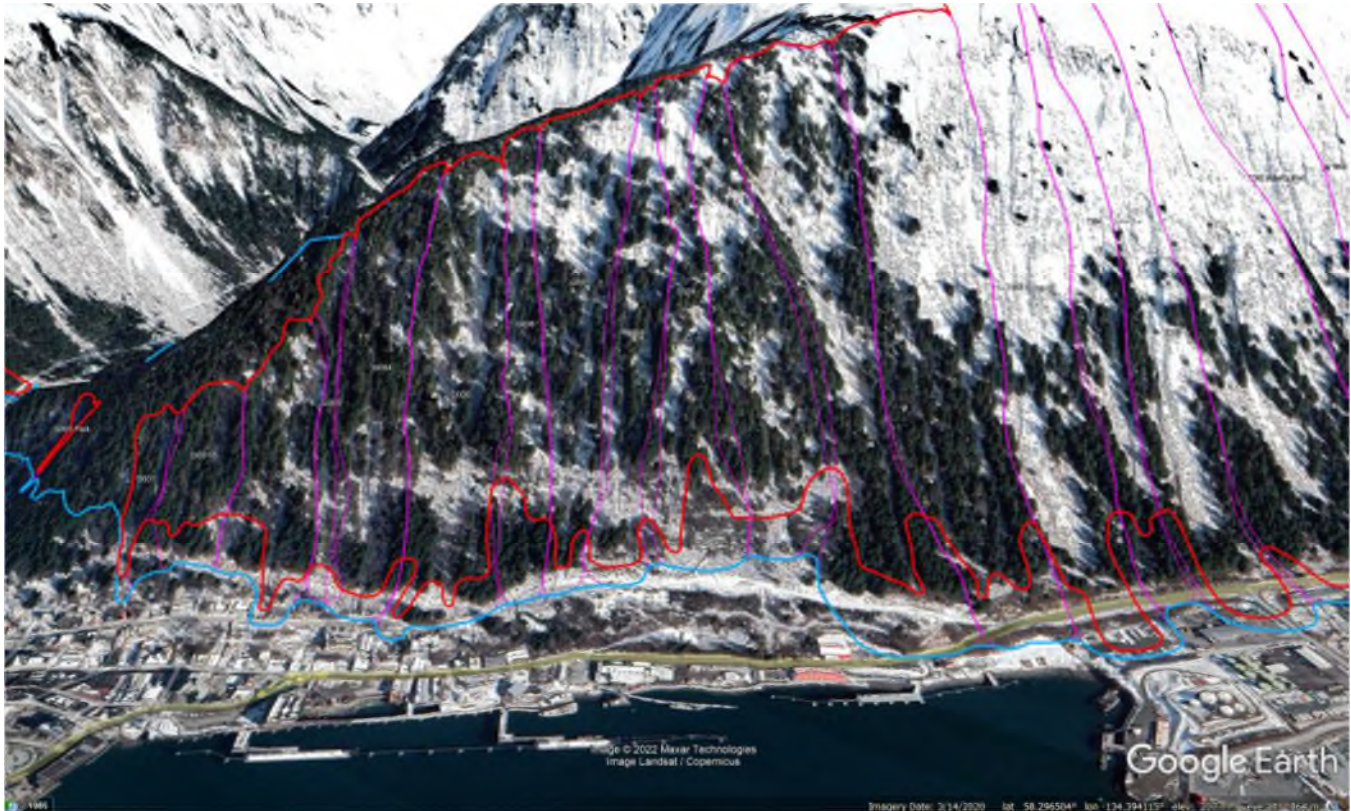
**Figure 3: Excerpt from Figure 2.4d in the main report, showing avalanche paths G003 to G009 and T000 (from left to right). The blue fringe shows terrain designated as having *Moderate* avalanche hazard along the toe of Mt. Roberts. In this area, the *Moderate* hazard does not reach South Franklin Street, but it does reach Gastineau Avenue in several locations. Further southeast (off the right-hand side of this map excerpt), the slopes of Mt. Roberts become higher and are affected by unforested alpine terrain, and the *Moderate* avalanche terrain reaches further downslope, past Thane Road, and sometimes into Gastineau Channel.**

Figure 4a below shows the slope from the helicopter, which is vegetated with a relatively dense forest cover in this area. Avalanche hazards are present within the gullied parts of the slopes, and have historically affected areas close to Gastineau Avenue. Figure 4b provides a view from Google Earth that shows distinct avalanche paths and start zones within the gullies that are easily seen on the winter imagery, which highlights the differences in coniferous versus deciduous forests.



**Figure 4a: View of Mt. Roberts from the helicopter showing part of the slope mapped in Figure 3. Note the increasing height of slope from left to right (northwest to southeast). The slope is fairly well-treed but is still prone to avalanching. Gullies tend to increase avalanche runouts. See also Figure 4b.**





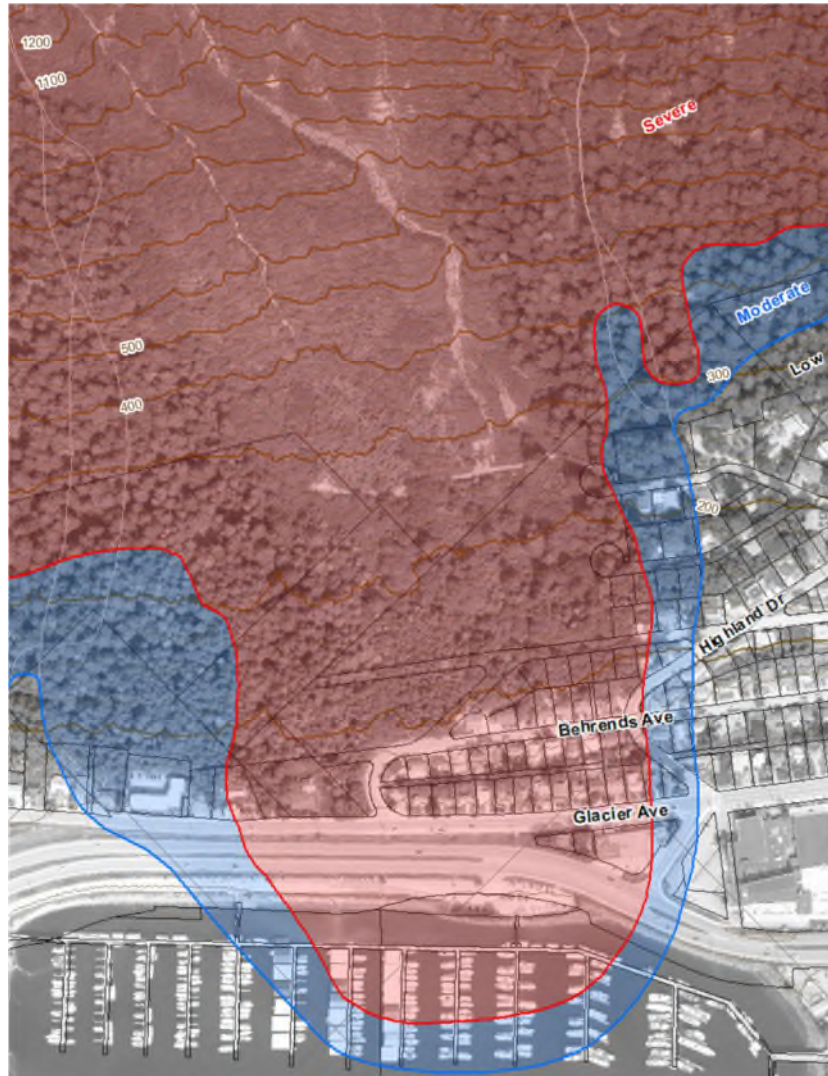
**Figure 4b: View of the northern part of Mount Roberts along Gastineau Avenue and South Franklin Street, with avalanche paths and *Moderate/Severe* hazard boundaries shown. Note the increasing height (and length) of slope from left to right (northwest to southeast), which increases the runout distance and hazard to lower elevation areas towards the industrial park. Although the Gastineau Avenue area is forested, distinct avalanche paths and start zones within the gullies can be observed on the winter imagery, which highlights the forest cover differences (coniferous versus deciduous forests). (Image credit: Google Earth 2022.)**

## 2.6 Avalanche Hazard Designation - Severe

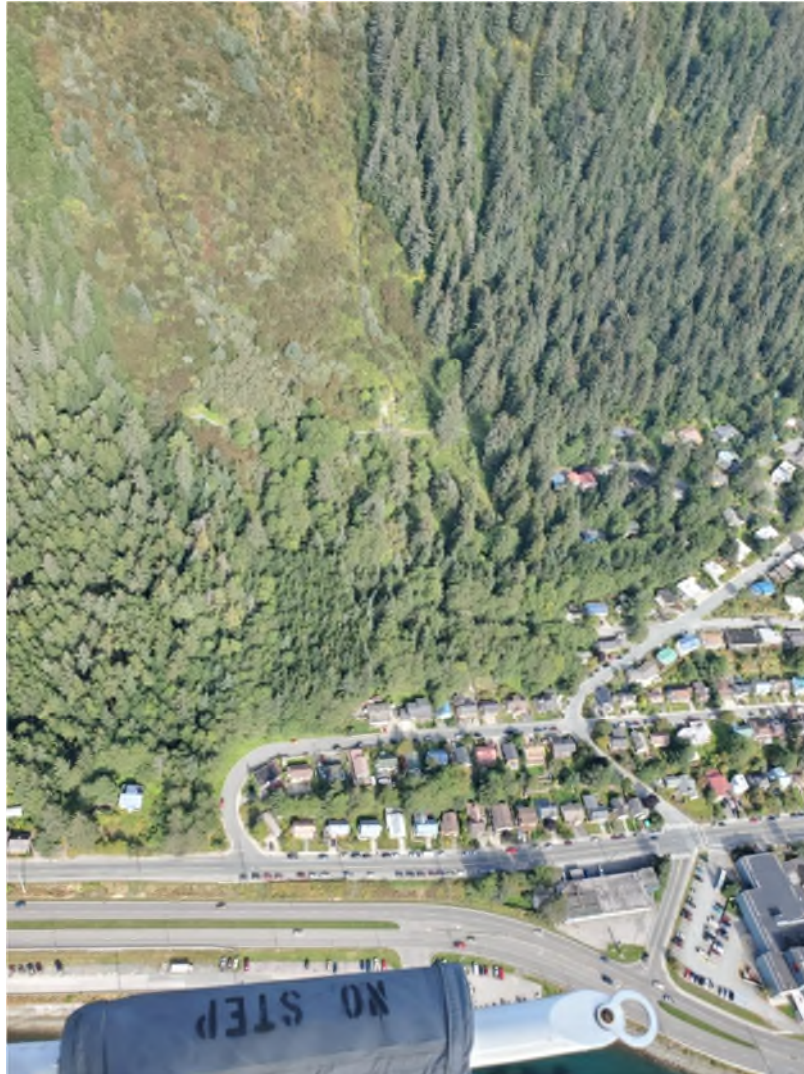
An avalanche hazard designation of *Severe* is used for avalanches that have a return period of less than 30 years AND/OR have an impact pressure greater than or equal to 600 lbs/ft<sup>2</sup> (30 kPa). Severe hazard areas could include areas that are affected by frequent, but lower impact pressure avalanche hazards, for example, an area that is affected on average every 5 to 10 years by avalanches with 200 lb/ft<sup>2</sup> to 400 lb/ft<sup>2</sup> (10 kPa to 20 kPa) impact pressures that could damage, but not destroy a wood-frame structure – this would be the case for some residential areas within the White Subdivision. Or it could include areas that, on average, are not affected by avalanches more frequently than at 30-year intervals but, should they be affected, would be impacted by large destructive avalanches with impact pressures well in excess of 600 lbs/ft<sup>2</sup> (30 kPa). This scenario applies to areas within the Behrends Subdivision. Although some parts of the subdivision have not been impacted since the large avalanche event of 1962 (e.g., some residences on Behrends Avenue), should a similar event occur within a 30- to 300-year return period, it would be expected to be large with impact pressures greatly exceeding 600 lbs/ft<sup>2</sup> (30 kPa). Areas that are affected by avalanches that are both frequent and destructive (i.e., less than a 30-year return period and with more than 600 lb/ft<sup>2</sup> of impact pressure) are clearly within the *Severe* hazard designation.

Avalanche areas mapped with a hazard designation of *Severe* are shown in red on the mapping. Typically, the *Severe* zone on the larger mountain slopes incorporates the main avalanche paths (mapped in red) that are the

most likely to experience an avalanche (i.e., higher frequency), and experience the highest impact pressures. In many cases within the Juneau area, this occurs within distinct gullies. Figure 5 is an example of this type of avalanche terrain in the Behrends avalanche path and subdivision. Figure 6 shows the slope from the helicopter. Figure 7 shows the lower part of the slope after a very large avalanche in 1985.



**Figure 5: Excerpt from Figure 2.4b showing the major avalanche path at J010 Behrends. Note the distinct trimlines that define the edges of this path, indicating regular avalanche activity within the central part of the path and less frequent avalanche activity on the outside (lateral boundaries) of the path.**



**Figure 6: Looking north-northeast at the lower end of the J010 Behrends Avenue avalanche path. Note the differences in vegetation within the path, beside the path, and below it, mostly due to regular and destructive avalanche activity. The large building in the lower right corner of the photo is the high school at the corner of Glacier Avenue and Highlands Drive. The school is located just outside of the *Moderate* hazard zone. Most of the other areas (with the exception of the densely forested upper right part of the photo) are within either *Severe* or *Moderate* hazard zones.**



**Figure 7: The aftermath of 1985 avalanche, looking north from just south of Behrends Avenue and Highland Drive. This event was the longest running avalanche in the Behrends Subdivision since the destructive 1962 event. The photo clearly shows the destructive potential of this avalanche and the way it came right into the community and was close to damaging/destroying many structures. However, it only damaged one house on Troy Avenue, the one on the right. (Photo credit: Dan Bishop 1985.)**

Figure 8 shows another avalanche event that occurred in 2012 further to the east of the Behrends Subdivision, at the Bathe Creek avalanche path.



**Figure 8: This photo illustrates a Size D3 avalanche within a Severe (red) hazard zone. The avalanche occurred in 2012 in the Bathe Creek avalanche path. This highlights a hazard area that is both frequent (more frequent than a 30-year return period) and destructive, with an impact pressure greater than 600 lbs/ft<sup>2</sup> (30 kPa), capable of both burying/destroying a car and destroying a wood frame residence. (Photo credit: Mike Janes (AELP).)**

## 3.0 LANDSLIDE HAZARD DESIGNATIONS AND DESCRIPTIONS

### 3.1 General

This section will provide:

- Definitions of the landslide hazard designations;
- Excerpts from the mapping to show examples of each designation;
- Photos with examples of the terrain in each of the hazard designations;
- Information on the difference in potential hazards from landslides above or below a property; and

- An explanation of the limitations of a hazards-only assessment.

## 3.2 What is a Landslide?

A landslide is a gravity-induced mass movement of upslope materials, including rockfall, rockslide, debris slide, debris flow, and creep. In general, landslide types include falls, topples, slides, spreads, flows, and slope deformations. Landslides can also contain broken trees, structures (whole or crushed), vehicles, or other materials, as well as water, in addition to soil and rock debris.

## 3.3 How are Landslide Hazards Designated?

Landslide hazard designations are determined based on collecting and reviewing previous mapping and reporting; historic landslide occurrence records including newspaper reports; air photos, satellite imagery, LiDAR data; mapping of surficial geology, historical slope movement activity, historical gully erosion activity; and fieldwork to confirm or correct the mapping.

The Downtown Juneau Study Area has been divided into areas with *Low*, *Moderate*, *High*, and *Severe* landslide hazard designations, according to the results of the historical air photo record analysis, mapping and the field investigation, as well as a semi-quantitative analysis to help sort out which terrain types belong to which landslide hazard designation. Areas mapped with *Low*, *Moderate*, *High*, and *Severe* landslide hazard designations are shown with green, yellow, orange, and red colours, respectively, in the mapping on Figures 1.6a through 1.6j, as well as Figure B.6 in Appendix B in the main report, and in the mapping excerpts shown in this memo. Table 4 provides a description of each hazard designation. Sections 3.4, 3.5, 3.6, and 3.7 provide some examples of the mapping for each hazard designation, and photos of the areas shown in the map excerpts. Table 4 in this memo is the same as Table 1.4 in the main report. This table includes some additional explanations of the typical sizes and event probabilities that would be anticipated for each of the landslide hazard designations. These same explanations are provided in the following sections for each level of hazard. These explanations are *not* based on a magnitude-frequency analysis for the slopes, because this type of analysis has not been completed for Juneau yet, as discussed in Section 5.0 of this Memo. Instead of a magnitude-frequency analysis, proxies based on slope activity identified on air photos were used to help determine the appropriate divisions between the different landslide hazard designations. The only landslide information considered reasonably reliable or predictable in attempting to determine typical return periods for each of the designations is the historical landslide information that has been reviewed, as listed at the beginning of this section. When results of a magnitude/frequency analysis are available, the return periods should be reviewed and adjusted as needed to more reliably reflect the frequency of landslides of particular sizes.

Note that sometimes the hazard is not related to what is happening right around your house, but what is happening higher on the slope or around your neighbour's house. That is especially true for hazards related to debris flows, because where the debris will end up is not always predictable. See also Technical Memo #2, Question #8 (Appendix C of the main report; Tetra Tech 2022b) for more information. Also, residents might not always know what happened to their lot or house before they moved there.

**Table 4: Refined Landslide Hazard Designation System**

Hazard Designation <sup>1</sup>	Symbol	Hazard Attribute Description
<i>Low</i>	L	<ul style="list-style-type: none"> <li>▪ Gentle to moderate slopes (0° to 26°)</li> <li>▪ No signs of historical landslide activity on the air photos</li> <li>▪ No written record of property damage or loss of life</li> <li>▪ Surficial geology and texture for Classes I, II, and III as shown in Table 1.2 in the main report</li> <li>▪ Estimated event probability is “Unlikely to Very Unlikely,” with a return period of more than 100 years. Class I, II, and III terrain is generally not prone to active slope processes, and no landslide events were observed or reported, so it is unlikely that landslide events would happen in the future<sup>2</sup></li> </ul>
<i>Moderate</i>	M	<ul style="list-style-type: none"> <li>▪ Moderate to Moderately steep slopes (27° to 35°)</li> <li>▪ May be signs of historical activity (scars on trees, vegetated debris lobes or scarps, historical activity visible on the air photos)</li> <li>▪ Can include low-lying areas within the runout zones of slides from nearby slopes</li> <li>▪ No apparent written record of property damage or loss of life</li> <li>▪ Surficial geology and texture for Class IV as shown in Table 1.2 in the main report</li> <li>▪ Estimated event probability is “Possible,” with a return period of 10 to 100 years. This is the return period estimated for Class IV terrain where slopes are susceptible to landslides, and where there might already be signs of landslide events. Therefore, landslide events could happen in the future<sup>2</sup></li> </ul>
<i>High</i>	H	<ul style="list-style-type: none"> <li>▪ Steep slopes (&gt;35°)</li> <li>▪ Areas where rockfall activity impacts individual trees but does not knock them over or destroy them<sup>3</sup></li> <li>▪ May have written record of property damage or loss of life</li> <li>▪ Surficial geology and texture for Class IV as shown in Table 1.2 in the main report</li> <li>▪ At least two of the following criteria are met:                             <ul style="list-style-type: none"> <li>– Thin layer of colluvium (Cv) present</li> <li>– A maximum polygon slope of 70° to 80°</li> <li>– A mean polygon slope of 40° to 50°</li> </ul> </li> <li>▪ Estimated event probability is “Likely,” with a return period of 5 to 30 years. This is the return period estimated for Class IV terrain where slopes are known to be susceptible to landslides, and where there are signs of recent and/or historical landslide events. Therefore, landslide events are likely to keep happening in the future<sup>2</sup></li> </ul>
<i>Severe</i>	S	<ul style="list-style-type: none"> <li>▪ Steep to vertical slopes (&gt;35°)</li> <li>▪ Signs of recent activity either in aerial photographs or from field inspection (rockfall tracks, debris slide activity, debris flow paths etc.)</li> <li>▪ May have written record of property damage or loss of life</li> <li>▪ Signs of repeated historical activity</li> <li>▪ Surficial geology and texture for Class V as shown in Table 1.2 in the main report</li> <li>▪ Estimated event probability is “Very Likely to Almost Certain,” with a return period of 1 to 20 years. This is the return period estimated for Class V terrain, where the slopes are highly susceptible to landslides, and where there are signs of recent landslide activity as well as repeated historical landslide activity. Therefore, landslide events are very likely to almost certain to keep happening in the future<sup>2</sup></li> </ul>

Notes:

1. Landslide hazard designations (*Low/Moderate/High/Severe*) correspond to green/yellow/orange/red on Figures 1.6a through 1.6j of the main report, and Figure B.6 in Appendix B of the main report.
2. Estimated event probability based on observed and recorded slope movement activity level. Note that this is not an indication of consequence (potential for damage), nor is it a magnitude/frequency study, which can determine return periods with more accuracy.
3. This type of rockfall can be highly active but has a small enough impact not to be readily visible on the air photos or satellite imagery.

Although the landslide hazard designations as shown in Table 4 do include a numerical figure to distinguish the estimated event probabilities of each of the landslide hazard designations, these very high-level approximations are based *only* on the observed slope movement activity levels from air photo analysis and observations made by Tetra Tech's geotechnical engineer in the field. In view of the information that is currently available, even more important are the other hazard attributes that help to better identify the types of terrain described by each hazard designation. For example, *Severe* hazard designations are assigned to the areas subject to rockfall, debris slides, and debris flows, as shown on the surficial geology maps. Areas with a *High* hazard rating were assigned based on the results of the semi-quantitative analysis. These areas are expected to experience rockfall that damages but does not always knock out trees, and as such are a less severe hazard than a debris flow or debris slide that removes everything in its path. Evidence of this type of rockfall activity was identified during the field investigation. See Sections 3.6 and 3.7 for more information about *High* and *Severe* hazard designations.

It should also be noted that the frequency or return period of an event (or the mapping proxy of visual evidence of repeated slide activity) does *not* mean that an event of a specified size or severity will return every X number of years. For example, a debris flow of a certain size typically depends on two events coinciding: a storm event large enough to mobilize debris in a gully, and enough debris accumulated in the gully from previous events to mobilize the debris. So, when a return period of 30 years is estimated for a rainstorm or a landslide, that means that a rainstorm or a landslide could happen at any time in a 30-year period, *not* that it will always happen every 30 years like clockwork. It could happen this year, and it could happen again next year. But if that rainstorm or landslide starts happening consistently more often (or less often) than predicted, so that the average is no longer 30 years, it might be time to reassess the return period for those events.

There are some important differences between the new hazard designation mapping and the adopted 1987 hazard designation mapping (CBJ 2021):

- The 1987 mapping and the current mapping have slightly different boundaries due to different project areas. These differences resulted in some areas being flagged as concerns, when the differences were in fact due to new areas being mapped that had not been mapped before (additional Study Area northwest and southeast, and to reach top-of-slope), or areas being omitted in the new mapping because they were beyond the top-of-slope boundary line of the new Study Area. Some areas were also inadvertently flagged as concerns, due to confusion resulting from the colour scheme used in the comparison, with the salmon pink being mistaken for red.
- The 1987 mapping combined avalanche and landslide hazard designations into one map. As it turns out, avalanche hazard designations and landslide hazard designations tend to be very different, and they should not be lumped together. The new maps show landslide and avalanche hazard designations on different maps, so that they can be managed independent of each other.
- The 1987 mapping follows property lines, resulting in numerous right-angle corners in the hazard boundaries. Landslides do *not* respect property lines, instead running right over them, and forming boundaries that relate only to the conditions that create landslides, such as slope gradients, topography, surficial geology, large storms (usually with record precipitation), rapid spring melt conditions, among other factors. The new landslide hazard designation mapping does not follow property boundaries, but rather reflects historical observations of landslide behaviour.
- *Due to these limitations, arbitrary hazard boundaries along property lines should be removed as not reflecting the true threat to the public safety, i.e., hazard designations based on property lines do **not** adequately describe the hazards.*

The level of assessment prepared for this project is suitable for determining whether land areas could be affected by landslides. A more detailed site-specific investigation and evaluation would be required to determine appropriate mitigations for specific properties.



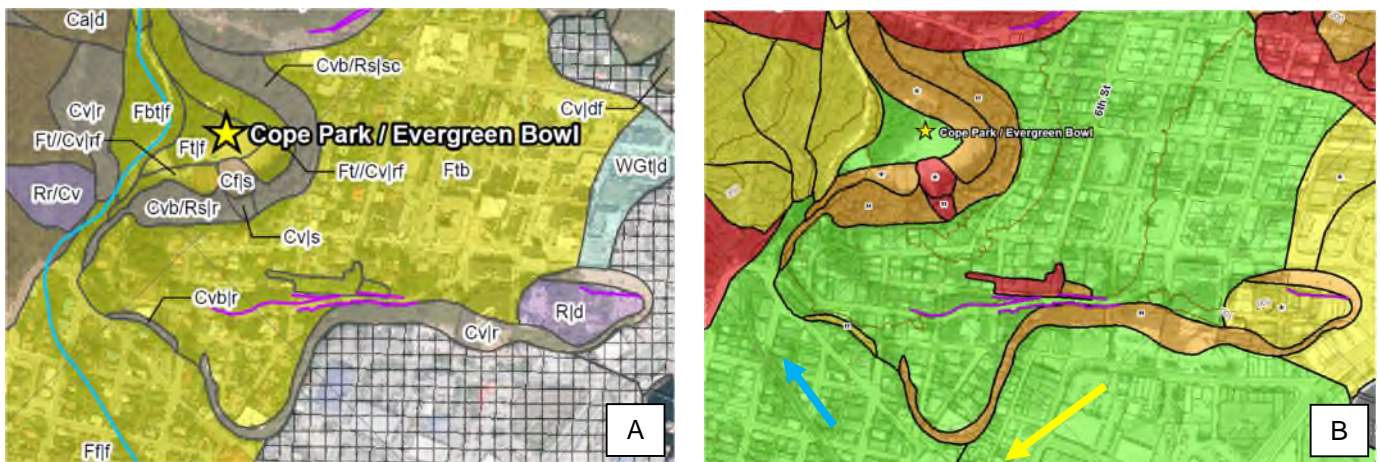
### 3.4 Landslide Hazard Designation - Low

A landslide hazard designation of *Low* is assigned to terrain that has the following characteristics:

- Gentle to moderate slopes ( $0^{\circ}$  to  $26^{\circ}$ );
- No signs of historical landslide activity on the air photos;
- No written record of property damage or loss of life;
- Surficial geology and texture for Classes I, II, and III as shown in Table 1.2 of the main report; and
- Estimated event probability is “Unlikely to Very Unlikely,” with a return period of more than 100 years. Class I, II, and III terrain is generally not prone to active slope processes, and no landslide events were observed or reported, so it is unlikely that landslide events would happen in the future.

Residents whose property is assigned a *Low* hazard designation, after never being in a named zone before, might wonder what that means. Including a *Low* hazard designation makes the mapping system consistent with numerous internationally accepted hazard mapping systems. This does not mean that the hazard has changed for properties that are now designated as being in a *Low* hazard zone. It just means that it has been given a name that recognizes that a hazard is never “zero,” but the hazard is low enough that owners of properties within the *Low* hazard zone generally should not have to do anything extra to protect their properties from landslides, except for being attentive, i.e., observing and recording anything unusual at or around their properties, such as ground settlement, cracking etc. The caveat to that logic could be if something changes around the property, like a structure being removed, or if the debris from a landslide was not recorded before it was cleaned up, making it difficult to detect where it occurred. Ideally, the mapping would be supported by good historical records, including property owner reporting, if applicable and available. See Question #9 on Tech Memo #3 for more information.

Figure 9 shows the surficial geology and the landslide hazard mapping for two areas of Downtown Juneau that are designated as having a *Low* landslide hazard. Figure 10 shows a photo for each of those areas.



**Figure 9:** These two map excerpts are from the mapping across the approximate middle of the downtown area. Figure 79A shows the surficial geology, and Figure 9B shows the landslide hazard mapping. Gold Creek is marked as a blue stream along the left side of Figure 9A. Willoughby Avenue is in the cross-hatched area on Figure 9A, where fill was placed to extend the land area of the city. The yellow arrow on Figure 9B shows the direction of look in Figure 10A. The blue arrow on Figure 9B shows the direction of look in Figure 10B.



**Figure 10: Views of Juneau in terrain mapped with landslide hazard designation of *Low*.**

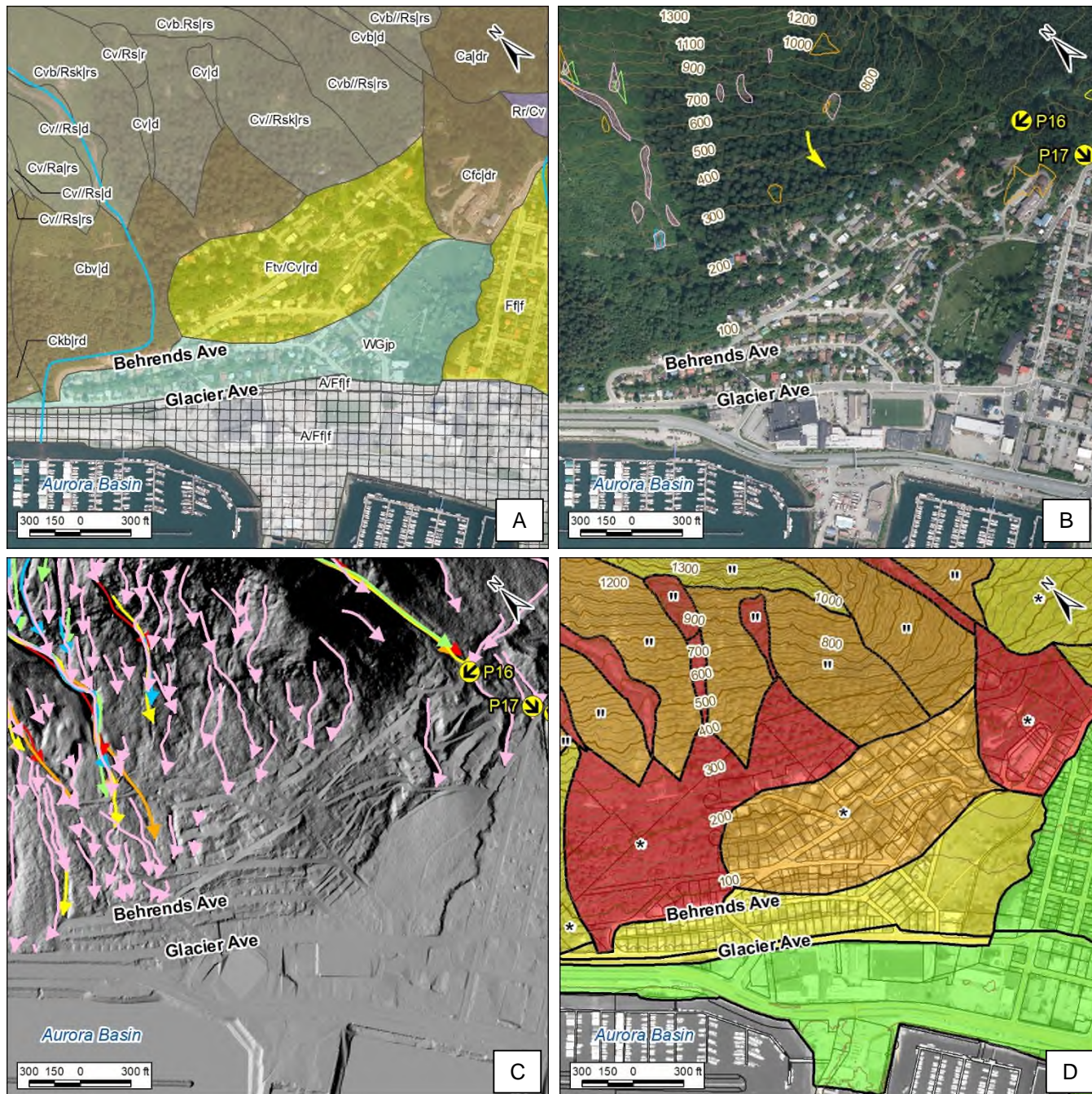
**Figure 10A: Looking west from Telephone Hill at the east-west leg of Willowby Avenue. Figure 10B: Looking upstream at the Gold Creek flume, with terrain mapped in *Low* on both sides of the creek. (Photo credits: Figure 10A: Alaska State Library – Historical Collections, [ASL-P417-040](#), Caroline Jensen 1948. ASL 2022a. Figure 10B: CBJ December 4, 2020.)**

### 3.5 Landslide Hazard Designation - *Moderate*

A landslide hazard designation of *Moderate* is assigned to terrain that has the following characteristics:

- Moderate to Moderately steep slopes (27° to 35°);
- May be signs of historical activity (scars on trees, vegetated debris lobes or scarps, historical activity visible on the air photos);
- Can include low-lying areas within the runout zones of slides from nearby slopes;
- No apparent written record of property damage or loss of life;
- Surficial geology and texture for Class IV as shown in Table 1.2 of the main report; and
- Estimated event probability is “Possible,” with a return period of 10 to 100 years. This is the return period estimated for Class IV terrain where slopes are susceptible to landslides, and where there might already be signs of landslide events or deposits from slides upslope. Therefore, landslide events could happen in the future.

Two sets of examples are provided for terrain designated with a *Moderate* landslide hazard: downslope of the Behrends Subdivision (Figures 11 and 13A), and downslope of South Franklin Street (Figures 12 and 13B).

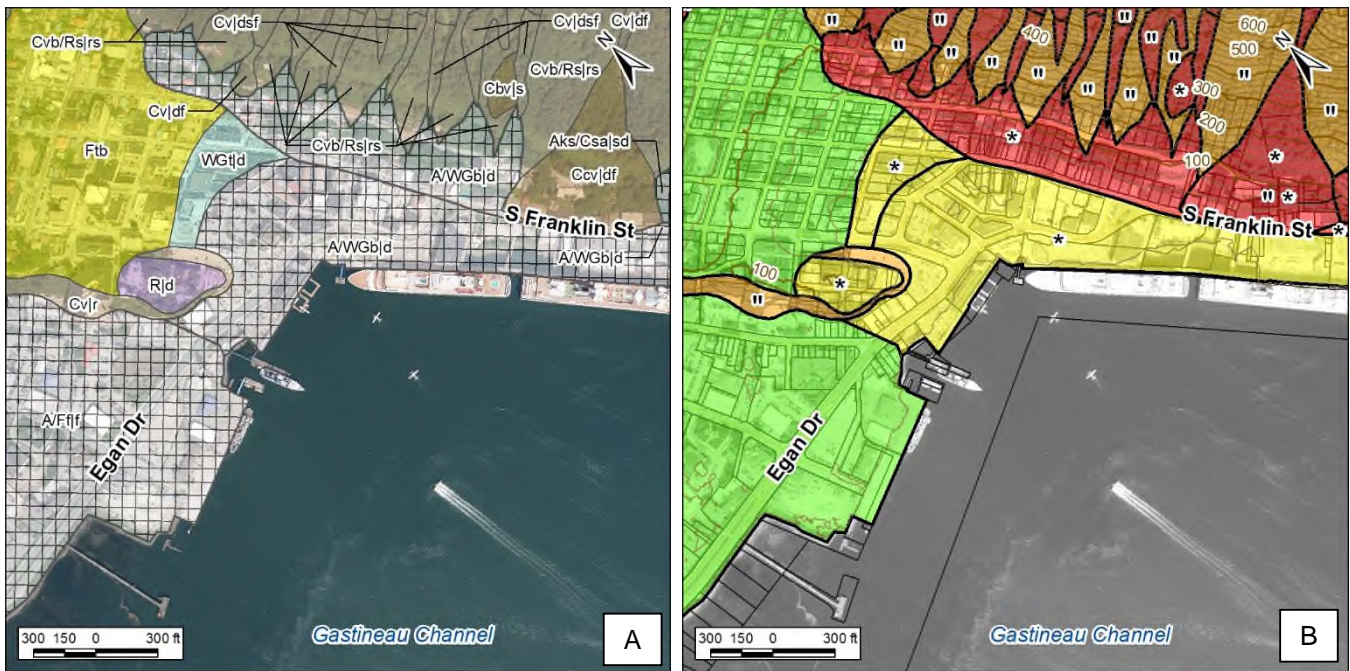


**Figure 11: These mapping excerpts show *Moderate* hazard terrain from part of the Juneau Study Area. Figures 11A through 11D. The terrain mapped as glaciomarine (WG) in Figure 11A reveals no slope movement features in Figure 11B, and no gully erosion features in Figure 11C. That results in a landslide hazard designation of *Moderate*, as shown on Figure 11D. See also Figure 13 for examples of *Moderate* terrain. Events that affect mainly roads (e.g., Figure 13A) tend to be cleaned up promptly and are generally not seen on the air photos.**

As shown on Figure 11D, Glacier Avenue at Ross Way is just below the *Severe* landslide designation zone at the Behrends Subdivision, at the northwest end of Behrends Avenue (left side of figure). That means Glacier Avenue could receive some smaller water-borne debris and muddy water that runs down the road, but it should not experience the more serious impacts generally seen further upslope. However, because some effects are still

possible, such as erosion (red arrow on Figure 13A), the landslide hazard designation here cannot be considered *Low*. As shown on Figure 11B, no other historical slope movement features were observed on the imagery, and on Figure 11C, the gullies appear not to extend across Behrends Avenue or Glacier Avenue, although the debris may flow onto them. Therefore, a landslide hazard designation of *Moderate* is considered appropriate.

Similar conditions apply downslope of South Franklin Street. The runouts of the several landslides on this slope are represented by surficial geology shown in Figure 12A and the *Severe* landslide hazard designations shown in Figure 12B.



**Figure 12:** These mapping excerpts show *Moderate* hazard terrain from another part of the Downtown Juneau Study Area. Figures 12A and 12B are from the Downtown Historic District, the east leg of Tideland, and the top of Telephone Hill. At center-left of Figure 12A, there is glaciomarine terrain (WG), and a rock outcrop (R) at Telephone Hill. Southeast of these two upslope areas, the terrain has been extensively human-modified (A). In this case, new ground was made from fill to create more space for the townsite development. These three areas together (except for the steep colluvial sideslopes of Telephone Hill) result in a *Moderate* hazard designation along the shoreline. In this case, a large landslide originating from the *Severe* terrain upslope of South Franklin Street (including the apparently mining-related events of 1920 and 1936) could have the potential to affect the *Moderate* terrain, but no obvious signs remain below South Franklin Street.

No mass movement events appear to have crossed South Franklin Street recently, and it is possible that the drainage and retaining structures erected along Gastineau Avenue could mitigate the extent of future landslides, at least at the 1920 landslide location. Nevertheless, the lower edge of the *Severe* terrain has been adjusted to be located downslope of South Franklin Street to account for the possibility of debris having been cleaned up and not seen on the imagery (Figure 12B). See Technical Memos #3, #6, and #7 (Appendix C in the main report; Tetra Tech 2022c, 2022f, 2022g) for more information about the slopes on Mt. Roberts.



**Figure 13: These photos are of *Moderate* terrain. Figure 13A is looking southeast along Glacier Avenue on December 4, 2020, where Ross Way enters. Ross Way carried debris and water from Behrends Avenue to Glacier Avenue. Debris can also run southeast on Behrends Avenue. Note the apparently eroded and failed section of the sidewalk (at red arrow) where a section of concrete slab was missing. Figure 13B is looking downslope towards South Franklin Street (formerly Front Street) on January 2, 1920, after a major landslide from upslope of Gastineau Avenue. The red circle shows possible landslide debris across the street. (Photo credits: Figure 13A: CBJ December 4, 2020. Figure 13B: Alaska State Library – Historical Collections, [ASL-P109-42](#), Katherine Shaw 1920. ASL 2022b.)**

On South Franklin Street (formerly Front Street), debris has sometimes crossed the road, for example, during the November 22, 1936 major landslide when debris reached the Juneau Cold Storage building, or as seems to have happened during the January 2, 1920 landslide, based on the photo in Figure 3B. However, these appear to be relatively rare events and, in the case of the 1920 landslide, seem to have been aggravated by a leaky flume from the Alaska Juneau Gold Mining Company (AJGMC) and, in 1936, was possibly aggravated by an oversteepened fill/spoil slope, also mining-related. The October 1, 1952 landslide resulted in debris blocking South Franklin Street.

Another major landslide on November 7, 1900 caused damage to a flume and the Juneau Iron Works building on the upslope side of South Franklin Street (Front Street), immediately southeast of where a later landslide on October 16, 1936 damaged the back of the Alaskan Hotel and destroyed several houses, and about 350 feet southeast of a landslide on September 25, 1918 that damaged the back of the Gastineau Hotel (now the New Cain Hotel) and destroyed several other buildings (Bayers 2022; Sanborn 1904, 1914; Swanston 1972; The Alaska Daily Empire 1918a).

Bayers also reported a “land & mud slide in the usual place back of the Manhattan Hotel, McMillan Bros. Grocery and Solomon the Tailor on S. Franklin St.” on November 7, 1918 (Bayers 2022; The Alaska Daily Empire 1918b, 1918c; Sanborn 1914). Those structures appear to have been located about where the Nor’Westerly, Frontier Gifts, and Tanzanite International are currently located, upslope of South Franklin Street.

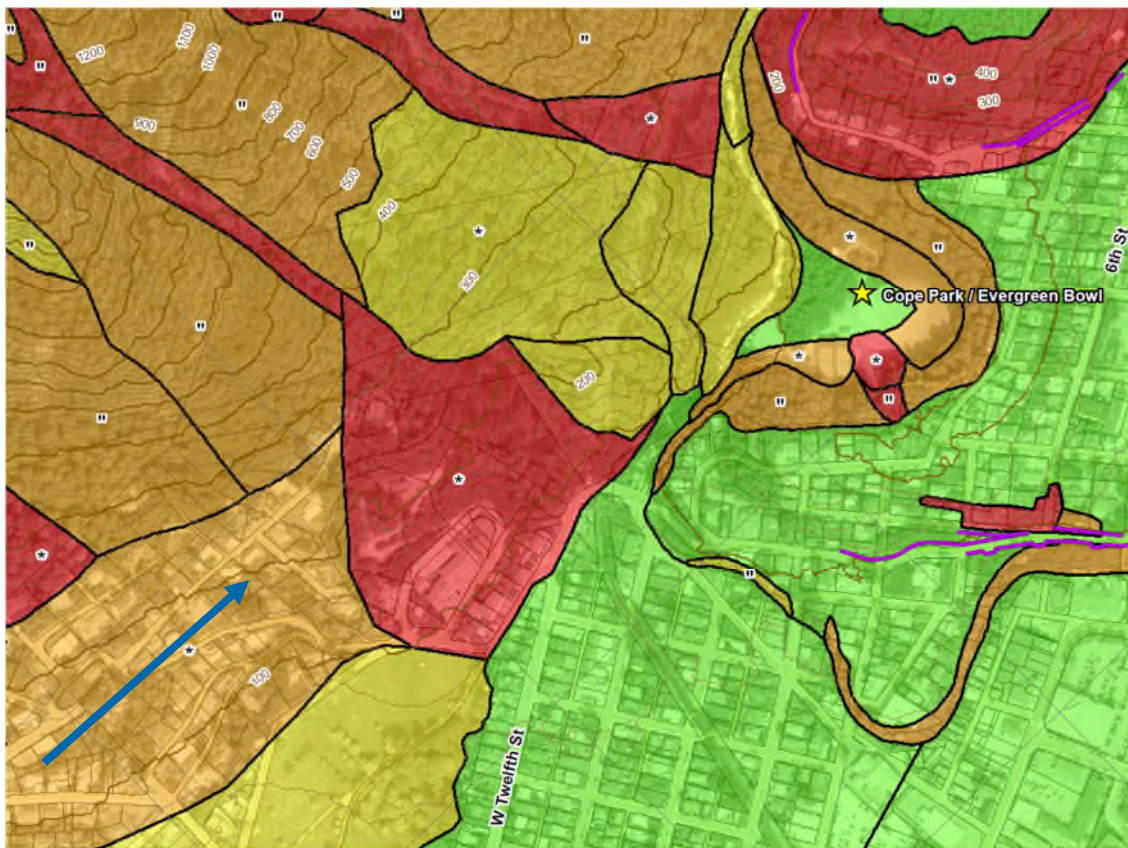
### 3.6 Landslide Hazard Designation - High

A landslide hazard designation of *High* is assigned to terrain that has the following characteristics:

- Steep slopes (>35°);
- Areas where rockfall activity impacts individual trees but does not knock them over or destroy them, resulting in an impact small enough not to be easily noticed on the air photos or satellite imagery;

- May have written record of property damage or loss of life;
- Surficial geology and texture for Class IV as shown in Table 1.2 of the main report;
- At least two of the following criteria are met:
  - Thin layer of colluvium (Cv) present;
  - A maximum polygon slope of 70° to 80°; and
  - A mean polygon slope of 40° to 50°.
- Estimated event probability is “Likely,” with a return period of 5 to 30 years. This is the return period estimated for Class IV terrain where slopes are known to be susceptible to landslides, and where there are also signs of recent and/or historical landslide events. Therefore, landslide events are likely to keep happening in the future.

Two example areas are provided for terrain designated with a *High* landslide hazard in the vicinity of Evergreen Avenue and around the slopes of Cope Park (Figures 14, 15, and 16).



**Figure 14: Excerpt from the landslide hazard mapping. Blue arrow on Figure 14 is direction of look on Figure 15, and back end of arrow is lower edge of photo in Figure 15. (See also Figures 9 and 11 for connecting map areas.)**



**Figure 15: View from the helicopter looking east towards Last Chance Basin (see direction of look on Figure 14). Upper Evergreen Avenue is approximately in line with direction of look.**

Most of the residential area in the foreground of Figure 15 is in *High* hazard zone. The upper ends of the road stubs in the foreground are mapped as *Severe* hazard (red arrows). Moving further east of the hairpin turn of Evergreen Avenue (further away from the camera), the upslope terrain is in *High* hazard zone (orange arrows) until the Bathe Creek fan/cone, where trees obscure the east end of Evergreen Avenue along the west edge of a large gully (near side outlined in red). The cemetery, which is the verdant green space at the lower right edge of the photo, is in *Moderate* hazard zone. The orange arrows at Cope Park (in the middle distance) show that most of the slope around the park is mapped as *High* hazard. See Figure 14 for more hazard mapping details. See Figure 16 for a close-up view of the slopes at Cope Park. See Technical Memo #2 (Appendix C in the main report; Tetra Tech 2022b) for more information about the Bathe Creek area.



**Figure 16: Looking southeast at the steep slopes around Cope Park at the ball diamond. Note the retaining wall at the toe of slope here, which is mapped as having a *High* hazard. (Photo credit: [CBJ Parks & Recreation 2022.](#))**

### 3.7 Landslide Hazard Designation - Severe

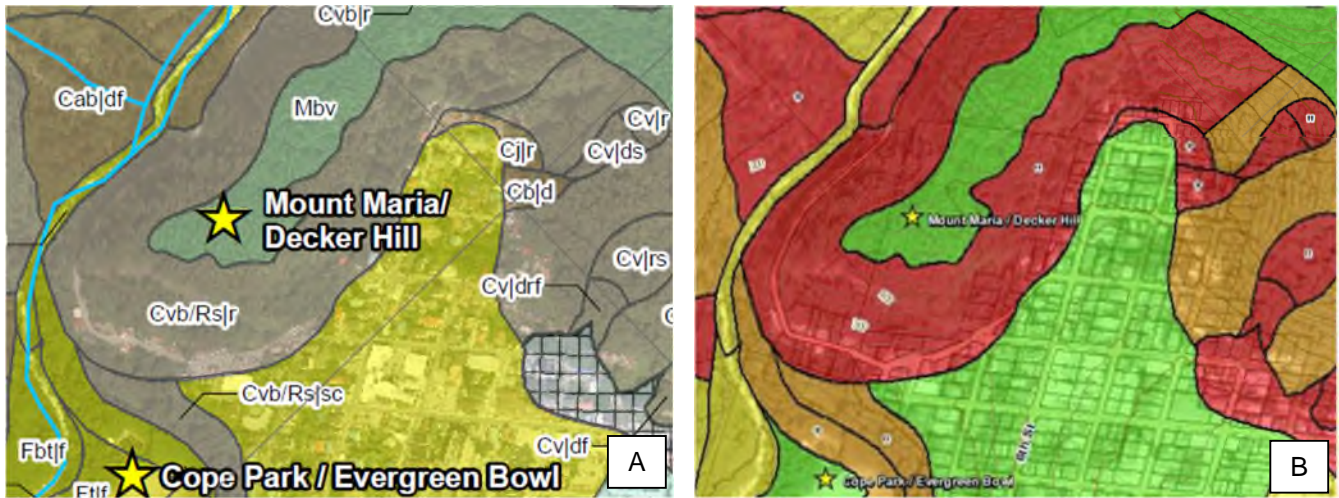
A landslide hazard designation of *Severe* is assigned to terrain that has the following characteristics:

- Steep to vertical slopes (>35°);
- Signs of recent activity either in aerial photographs or from field inspection (rockfall tracks, debris slide activity, debris flow paths etc.);
- May have written record of property damage or loss of life;
- Signs of repeated historical activity;
- Surficial geology and texture for Class V as shown in Table 1.2 of the main report; and
- Estimated event probability is “Very Likely to Almost Certain,” with a return period of 1 to 20 years. This is the return period estimated for Class V terrain, where the slopes are highly susceptible to landslides, and where



there are signs of recent landslide activity as well as repeated historical landslide activity. Therefore, landslide events are very likely to almost certain to keep happening in the future.

Two sets of examples are provided for terrain designated with a *Severe* landslide hazard: at the northeast end of the Starr Hill subdivision, above Nelson Street (Figures 17 and 18), and at the northwest end of the White Subdivision (Figures 19 and 20). As these examples show, *Severe* landslide hazards can occur on relatively short slopes or on very long slopes.

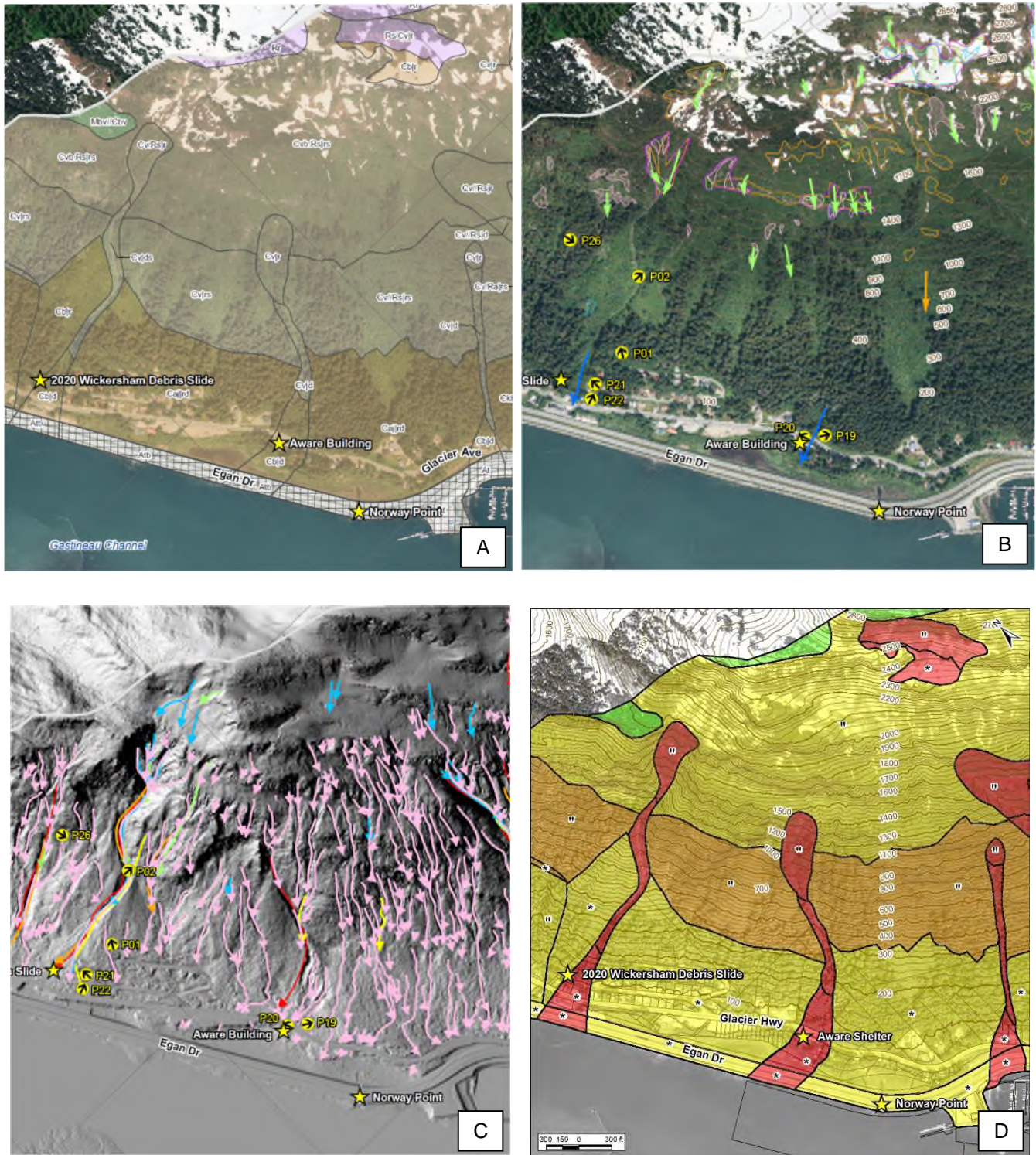


**Figure 17: These two map excerpts are from the mapping slopes above Starr Hill. Figure 17A shows the surficial geology, and Figure 17B shows the landslide hazard designation mapping. Around Starr Hill, the green signifies *Low* hazard, the orange is *High* hazard, and the red is *Severe* hazard. See Figure 18 for the landslide seen in the *Severe* hazard area above Nelson Street. More information about this area is available in Technical Memo #3.**

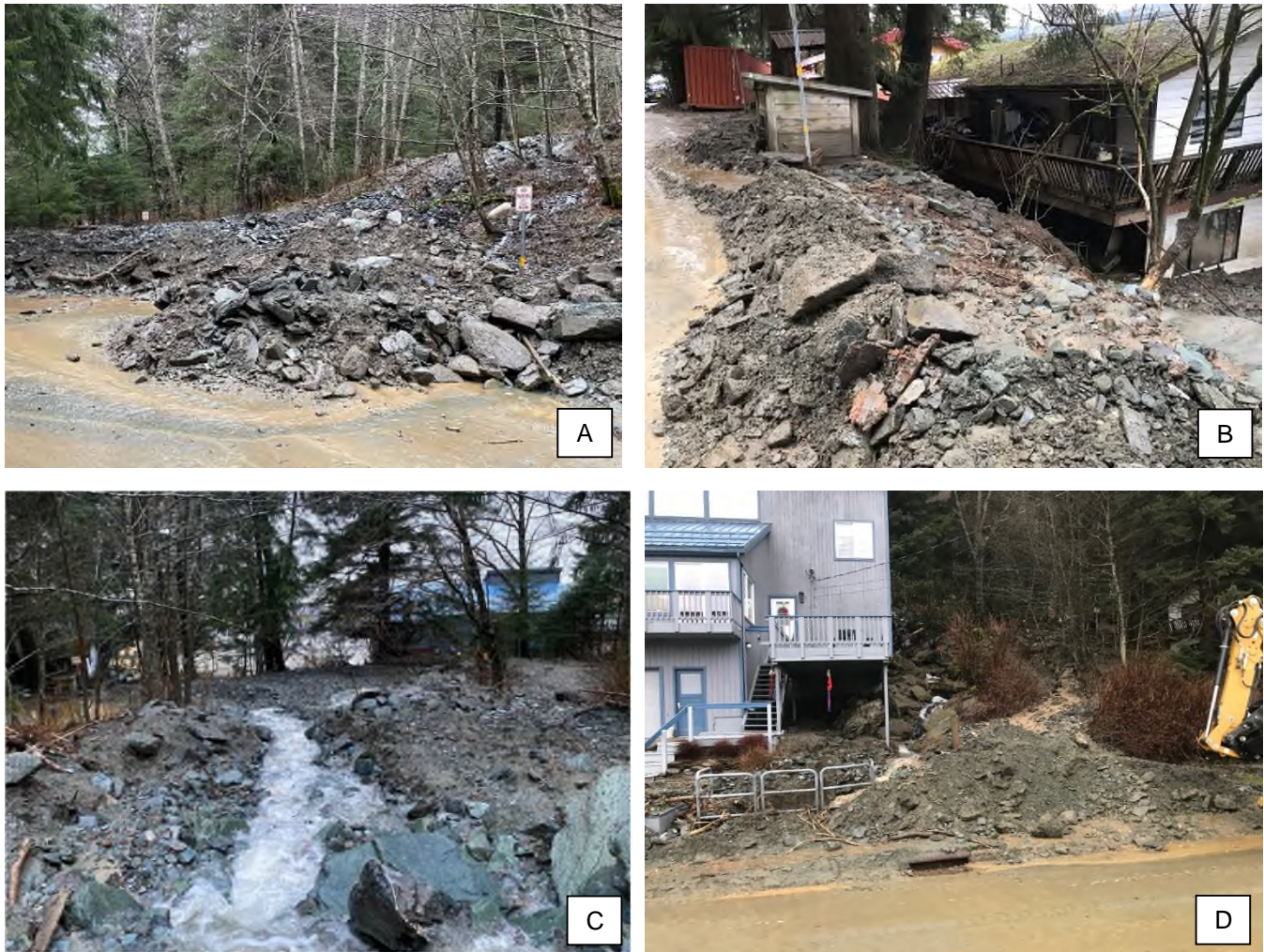
Figures 17B and 19D are hazard maps, which indicate areas that are potentially hazardous. If there was a lot of potentially hazardous geomorphic process activity on a slope, or if new activity was identified in the field, that area was mapped as having a *Severe* hazard. For instance, debris could be building up on the slope directly above a house (Figure 18), or in a location where debris can potentially run towards a house, and where it could become a more serious hazard in the future (Figures 20B and 20D). Smaller debris slides and debris flows tend to accumulate debris material in wedges within gullies. Eventually, when a critical level of debris accumulation is reached, or a significant precipitation event occurs, all that stored debris is scoured out of the gully, potentially resulting in a very large debris flow event. Similar events can occur on open slopes where slide debris piles up in lobes over days, months, or years, sometimes separated by channels of faster-flowing loose material. These debris lobes can slowly be creeping downslope, until the critical moment when there is enough mass and enough water to trigger the debris flow rapidly downslope. See also Technical Memo #2 for more mapping examples (Appendix C in the main report; Tetra Tech 2022b).



**Figure 18: Compares Tetra Tech’s photo from September 10, 2019 (Figure 18A) with residents’ photo from August 1, 2021 (Figure 18B) at the same location. Slope instabilities appear to be ongoing in the historical slide paths located above Nelson Street on a slope with a landslide hazard designation of Severe. See Technical Memo #3 for more information about the slopes around Starr Hill.**



**Figure 19: These excerpts are from the mapping at the slopes at the White Subdivision. Figure 19A shows the surficial geology, Figure 19B shows the slope movement features, Figure 19C shows the gully erosion features, and Figure 19D shows the landslide hazard mapping. The Wickersham slide (Figure 20) is related to a very active gully erosion feature in Severe hazard.**



**Figure 20: The Wickersham debris slide in the White Subdivision is an example of a landslide in terrain with a Severe landslide hazard designation. Figure 20A: Part of the debris deposit at the northwest end of Wickersham Avenue on the uphill side; Figure 20B: Debris on the downhill side of Wickersham Avenue; Figure 20C: Debris running down along the swale between Wickersham and Glacier Highway; Figure 20D: Debris deposit at Glacier Highway, filling a concrete sump behind the railing. (Photo credits: CBJ December 4, 2020.)**

Figure 20 shows the aftermath of a large debris slide, after the roads had been mostly cleared. The debris was up to 8 feet thick at Wickersham Avenue, crossing Wickersham to impact a residence, filling a drainage path to Glacier Avenue, filling a drainage sump, and flowing out onto Glacier Avenue. By the time the photo in Figure 20C was taken, the water was running clear again. More debris is visible on the right, where it ran down to Glacier Highway.

## 4.0 HAZARD FROM ABOVE OR HAZARD FROM BELOW

Landslide hazards can affect properties from both upslope and downslope. Landslide hazards that affect properties from upslope are landslides that have the potential to run down a slope and impact a property, overrun it, damage it, or destroy it. Landslide hazards that affect properties from downslope are landslides that have the potential to remove part of a property when the ground falls downslope away from the property. For example, part of the backyard falls down the hill, or so much ground falls away that the foundation of the building is endangered. The

worst case would be if so much ground falls away that the building can no longer be supported, and it too will topple or slide downhill.

A few examples of areas of Juneau where landslide hazards from above can potentially affect property include Tidelands, Starr Hill, Gastineau Avenue, Behrends, Highlands, and the White Subdivision. A few examples of areas where landslide hazards from below can potentially affect property include Chicken Ridge, Telephone Hill, and the northwest corner of Juneau Townsite (as shown on the [Historical Neighborhoods](#) website (CBJ 2022)). Chicken Ridge is also the main area where landslides can affect property from both above and below, for example, along Basin Road, and in a few places along Goldbelt Avenue.

## 5.0 LIMITATIONS OF A HAZARDS-ONLY ASSESSMENT

A detailed risk assessment would generally include the following basic steps:

- Hazard assessment;
- Magnitude/frequency analysis;
- Consequence assessment; and
- Risk assessment.

Depending on the requirements of the project, more data is acquired to satisfy each of the steps. The Downtown Juneau Landslide and Avalanche Hazard Assessment project has completed the first step – the hazard assessment. The other three steps were not part of the scope for this project. The thorough hazard assessment completed by Tetra Tech (Tetra Tech 2021, 2022) provides important information on where the past, present, and future slope instability areas are located in Downtown Juneau. This information can be used to progress to the other three steps.

Future phases of the project would allow more information to be collected and analysed, but each task also requires considerably more work and funding to acquire the necessary data before each subsequent task can be completed. See Technical Memo #1 for more information (Appendix C of the main report; Tetra Tech 2022a).

For example, the magnitude/frequency analysis would allow the slope activity data to be refined so that it could be used to help predict return periods for landslides of a specific type and size for a particular site, like a debris flow gully. Consequences could then be evaluated. For instance, if a specific gully experiences debris flows, i.e., acts as a conduit for conveying debris downslope, what happens downslope if it is only a small debris flow? What happens if it is a very large debris flow? Maybe nothing happens, because there are no buildings below, or maybe several buildings are destroyed when the debris runs into them.

Finally, a risk assessment can be done with a combination of all the data gathered in the previous steps. Land management decisions can then be made based on what is considered to be a tolerable risk, such as having to occasionally clean debris off the road; or what is considered to be an intolerable risk, such as a debris slide overrunning a house with someone in it.

The main challenge for CBJ at present is managing questions that require a risk assessment to be answered satisfactorily when the only data available so far are the results of the hazard assessment (Tetra Tech 2021).

## 6.0 REQUESTS FOR ADDITIONAL INFORMATION

A few specific questions were asked and are addressed specifically in this section. With the background information provided in the previous sections, the reader will understand the context of the answers. With limited data, it is not always possible to find a complete answer, but it will also help to understand what the landslide hazard designations mean when describing what could happen.

- **Question:** Does a *Severe* landslide hazard designation mean it would be a catastrophic failure?

**Answer:** A *Severe* landslide hazard designation only describes the hazard. A description of the hazard can include information like the type of landslide (debris slide, debris flow, rockfall etc.), the size, and the location. If there is lots of data, such as many years of air photos, satellite imagery, cleanup reports, damage reports, that helps to give an idea of landslide activity and size. That is, out of 10 historical air photos of a particular slope taken over 70 years, does a landslide scar appear only once? Twice? Every year that is checked? How large is the area affected? How much debris needs to be cleaned up? Which structures are damaged and where are they located?

A *Severe* landslide hazard designation does *not* specifically mean a catastrophic failure. In the case of this study, there are two main criteria that are used to decide whether an area needs to be designated as *Severe*:

- Evidence of slope instability within the same feature in more than one air photo or LiDAR year and/or field investigation year; and/or
- A cone or fan of colluvium is present at the base of a slope, no matter how old it is, because the hazard is still present.

Numerous gullies in Juneau show evidence of slope instabilities in several years (sometimes every year) of imagery, incident report data, or field observation data that was reviewed.

More steps are needed to determine whether a landslide in an area designated *Severe* would be catastrophic or not. One of the most important steps would be a consequence assessment, summarized in Section 6.0. See Question #1 in Technical Memo #2 for more information on how a *Severe* landslide hazard designation is determined (Appendix C of the main report; Tetra Tech 2022b).

- **Question:** What about the *Moderate* areas of the Highlands and Downtown Juneau – are they low probability, high consequence? Wouldn't any landslide damage be catastrophic?

**Answer:** A *Moderate* landslide hazard designation only describes the hazard; it does not describe the consequence. Estimating the probability of a landslide requires a magnitude/frequency analysis. Evaluating the consequence of a landslide requires a consequence analysis. Neither of those tasks was in the scope and they not been done.

However, let's compare the different landslide hazard designations shown in Table 1.4 in Section 3.0 above. The description for a *Moderate* landslide hazard might be somewhat reassuring compared to the description for *High* or *Severe* landslide hazards. Since there is insufficient data to determine a return period for a possible landslide of a particular size, the only basis for comparison is to consider the other characteristics of the designation. To summarize, landslides are possible, and there might (or might not) be signs of past landslides, but there is no apparent record of damage or loss of life.

Although the natural terrain in some parts of Juneau has been obscured by construction-related earthworks, very large events in the past have left traces, like the very large prehistoric landslides mapped along the valley

slopes (Swanston 1972). In contrast, the large suspected deep-seated bedrock failure southeast of Snowslide Creek is rated *Severe*, even though it has not yet happened. Despite these exceptions, even if a landslide happens only rarely, it does not necessarily mean that a rare event is always going to be the “big one.” Conceivably, land managers could decide to avoid all areas in which a landslide could occur, including those with a designation of *Moderate*, but the priority should be to avoid the *Severe* and *High* designated areas first, because those areas will usually be affected more often and more seriously than the *Moderate* ones.

See also Section 3.5 for examples of *Moderate* terrain and mapping.

- **Question:** Can you provide additional explanatory terms that reference a general timeframe for a specific landslide hazard designation, e.g., *Low* – geologic time, *Moderate* – 100 to 1,000 years etc.?

**Answer:** Without a magnitude/frequency analysis, it is not possible to definitively tie the landslide hazard designation to a specific timeframe. The activity level observed during the historical air photo record analysis and the fieldwork, as well as occasional reported events, provide the only information about frequency that is currently available. The activity level does have some correlation to frequency (i.e., more active landslide areas experience landslides more frequently), but that is not the same as having the results of a more rigorous magnitude/frequency analysis. Based on the activity levels, it is only possible to tie the landslide hazard designations to a much shorter timeframe, as described in Section 3.0.

- **Question:** Can you tell me more about the proxies that are being used instead of a magnitude/frequency analysis?

**Answer:** A useful proxy for magnitude is the size of the unvegetated slope area (or range of sizes), based on the typical sizes of the events seen on the available air photos, satellite images, and evidence seen during the field work. Another proxy for magnitude is whether any damage or loss of life was reported for a specific landslide event. (In risk studies – *not* part of the current scope – reports of size, damage or loss of life would also contribute to an understanding of consequence.)

The proxy for frequency is activity: the proportion of air photo or satellite images (or field observations) that show a lack of vegetation on a slope that would ordinarily be vegetated. The more often a slope section or gully has no vegetation on it, the higher the rating it will receive. Areas showing activity in two or more air photo years were identified and given a hazard designation of *Severe* on the hazard designation maps due to their higher activity levels. In fact, many of the areas designated as *High* or *Severe* in the mapping turned out to have several instances of lack of vegetation, with numerous gullies showing evidence of slope instabilities for all, or almost all, observation dates. See Section 3.3 in this memo, and additional discussion in the answer to Question #1 in Technical Memo #2 (Appendix C of the main report; Tetra Tech 2022b).

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## 8.0 CLOSURE

We trust this technical memo meets your present requirements. If you have any questions or comments, please contact the undersigned.

Respectfully submitted,  
Tetra Tech Canada Inc.



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## GEOTECHNICAL

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The Professional Document is not applicable to any other sites, nor should it be relied upon for types of development other than those to which it refers. Any variation from the site conditions present, or variation in assumed conditions which might form the basis of design or recommendations as outlined in this report, at or on the development proposed as of the date of the Professional Document requires a supplementary investigation and assessment.

TETRA TECH is neither qualified to, nor is it making, any recommendations with respect to the purchase, sale, investment or development of the property, the decisions on which are the sole responsibility of the Client.

## 1.7 ENVIRONMENTAL AND REGULATORY ISSUES

Unless stipulated in the report, TETRA TECH has not been retained to investigate, address or consider and has not investigated, addressed or considered any environmental or regulatory issues associated with development on the subject site.

## 1.8 NATURE AND EXACTNESS OF SOIL AND ROCK DESCRIPTIONS

Classification and identification of soils and rocks are based upon commonly accepted systems and methods employed in professional geotechnical practice. This report contains descriptions of the systems and methods used. Where deviations from the system or method prevail, they are specifically mentioned.

Classification and identification of geological units are judgmental in nature as to both type and condition. TETRA TECH does not warrant conditions represented herein as exact, but infers accuracy only to the extent that is common in practice.

Where subsurface conditions encountered during development are different from those described in this report, qualified geotechnical personnel should revisit the site and review recommendations in light of the actual conditions encountered.

## 1.9 LOGS OF TESTHOLES

The testhole logs are a compilation of conditions and classification of soils and rocks as obtained from field observations and laboratory testing of selected samples. Soil and rock zones have been interpreted. Change from one geological zone to the other, indicated on the logs as a distinct line, can be, in fact, transitional. The extent of transition is interpretive. Any circumstance which requires precise definition of soil or rock zone transition elevations may require further investigation and review.

## 1.10 STRATIGRAPHIC AND GEOLOGICAL INFORMATION

The stratigraphic and geological information indicated on drawings contained in this report are inferred from logs of test holes and/or soil/rock exposures. Stratigraphy is known only at the locations of the test hole or exposure. Actual geology and stratigraphy between test holes and/or exposures may vary from that shown on these drawings. Natural variations in geological conditions are inherent and are a function of the historic environment. TETRA TECH does not represent the conditions illustrated as exact but recognizes that variations will exist. Where knowledge of more precise locations of geological units is necessary, additional investigation and review may be necessary.

## 1.11 PROTECTION OF EXPOSED GROUND

Excavation and construction operations expose geological materials to climatic elements (freeze/thaw, wet/dry) and/or mechanical disturbance which can cause severe deterioration. Unless otherwise specifically indicated in this report, the walls and floors of excavations must be protected from the elements, particularly moisture, desiccation, frost action and construction traffic.

## 1.12 SUPPORT OF ADJACENT GROUND AND STRUCTURES

Unless otherwise specifically advised, support of ground and structures adjacent to the anticipated construction and preservation of adjacent ground and structures from the adverse impact of construction activity is required.

## 1.13 INFLUENCE OF CONSTRUCTION ACTIVITY

There is a direct correlation between construction activity and structural performance of adjacent buildings and other installations. The influence of all anticipated construction activities should be considered by the contractor, owner, architect and prime engineer in consultation with a geotechnical engineer when the final design and construction techniques are known.

## 1.14 OBSERVATIONS DURING CONSTRUCTION

Because of the nature of geological deposits, the judgmental nature of geotechnical engineering, as well as the potential of adverse circumstances arising from construction activity, observations during site preparation, excavation and construction should be carried out by a geotechnical engineer. These observations may then serve as the basis for confirmation and/or alteration of geotechnical recommendations or design guidelines presented herein.

## 1.15 DRAINAGE SYSTEMS

Where temporary or permanent drainage systems are installed within or around a structure, the systems which will be installed must protect the structure from loss of ground due to internal erosion and must be designed so as to assure continued performance of the drains. Specific design detail of such systems should be developed or reviewed by the geotechnical engineer. Unless otherwise specified, it is a condition of this report that effective temporary and permanent drainage systems are required and that they must be considered in relation to project purpose and function.

## 1.16 BEARING CAPACITY

Design bearing capacities, loads and allowable stresses quoted in this report relate to a specific soil or rock type and condition. Construction activity and environmental circumstances can materially change the condition of soil or rock. The elevation at which a soil or rock type occurs is variable. It is a requirement of this report that structural elements be founded in and/or upon geological materials of the type and in the condition assumed. Sufficient observations should be made by qualified geotechnical personnel during construction to assure that the soil and/or rock conditions assumed in this report in fact exist at the site.

## 1.17 SAMPLES

TETRA TECH will retain all soil and rock samples for 30 days after this report is issued. Further storage or transfer of samples can be made at the Client's expense upon written request, otherwise samples will be discarded.



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<b>To:</b>	Teri Camery (CBJ)	<b>Date:</b>	April 27, 2022
<b>c:</b>	Scott Ciambor (CBJ)	<b>Memo No.:</b>	5
<b>From:</b>	Rita Kors-Olthof, Vladislav Roujanski	<b>File:</b>	704-ENG.EARC03168-02A
<b>Subject:</b>	Landslide Hazard Designations at Telephone Hill and Gastineau Avenue Downtown Juneau Landslide and Avalanche Hazard Assessment		

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## 1.0 INTRODUCTION

Tetra Tech Canada Inc. (Tetra Tech) has prepared an Issued-for-Review (3<sup>rd</sup> Draft) Report, Downtown Juneau Landslide and Avalanche Assessment for the City and Borough of Juneau (CBJ), dated May 28, 2021 (Tetra Tech 2021); and participated in three Landslide and Avalanche Hazard Public Meetings that took place on July 21, August 10, and September 20, 2021.

Following CBJ's initial email request of July 27, 2021, Tetra Tech responded to comments and questions that arose from the July 21, 2021, Public Meeting with a series of three technical memos. These memos were Issued-for-Review to CBJ, along with an email providing supplemental information, and have since been updated (Appendix C of the main report; Tetra Tech 2022a, 2022b, 2022c).

CBJ has now requested a further series of memos to address additional landslide hazard-related questions, as well as a review of historical avalanche data, to address further questions that arose following the August 10 and September 20, 2021, Public Meetings; as well as some follow-up questions from CBJ. The scope is as described in Tetra Tech's proposal of December 9, 2021, with a few modifications as discussed during the kick-off meeting with CBJ on February 8, 2022. All the completed memos will be appended to the Final Draft Report.

This Technical Memo #5 provides some additional explanation for anticipated future slope instabilities within the landslide hazard designations mapped as *High* or *Severe* on the slopes of Telephone Hill (Figures 1 and 2) compared to the areas mapped as *High* or *Severe* on the slopes along and above Gastineau Avenue.

## 2.0 SCOPE AND METHODS

The primary objective of this memo is to address the question, "The area of Telephone Hill and the bluffs below is mapped as a *High* hazard. What is the difference between Telephone Hill and the steep slopes on Gastineau in terms of hazard and potential for damage?" Specific tasks included the following:

- Review landslide hazard designation mapping completed by Tetra Tech;
- Locate suitable photographs illustrating landslide hazards in the above-noted map areas, if/as needed;
- Prepare map excerpts, if/as needed;
- Refer to information presented previously in other technical memos, as applicable; and
- Prepare Technical Memo, providing descriptions and/or comparisons, as needed.

The surficial geology mapping shows that the colluvial terrain at Telephone Hill is connected to the northwest leg of the Juneau townsite, which is in turn connected to the northwest leg of Chicken Ridge on the southwest side of Cope Park. Therefore, it is logical to consider these areas together in addressing this question.

### 3.0 TELEPHONE HILL, JUNEAU TOWNSITE, AND CHICKEN RIDGE

#### 3.1 Summary of Historical Landslides in Areas Mapped *High or Severe*

It is useful to first consider a view of Telephone Hill from Mt. Maria in the historical photo taken in about 1896, prior to the Juneau development that has gradually obscured the slopes (Figure 1). The prominent bedrock ridge seen in this photo was mapped by Miller (1975) as undifferentiated Tertiary or upper Mesozoic rock with an unconformity, suggesting that some material was scoured off at some point in geological history, and the bottom of the upper layers does not match the top of the lower layers. Although the top of the ridge is gently sloped, the sides of the ridge are quite steep. In locations where a thin veneer of colluvium covers the bedrock, this material could be more prone to mass movement than other materials such as a blanket of colluvium or glacial till (Tetra Tech 2021a).



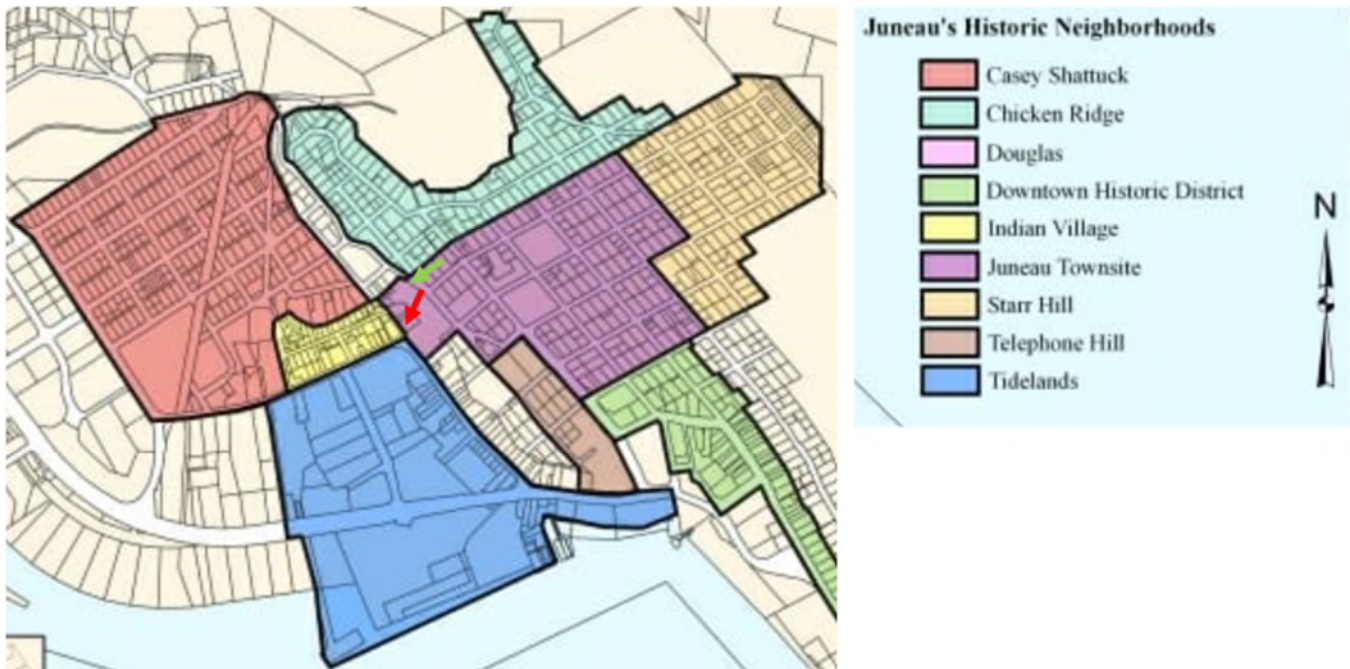
**Figure 1: Looking south towards Telephone Hill and Gastineau Channel from Mt. Maria, circa 1896, early in the development of Juneau, when structures along the toe of slope at Willoughby Avenue were supported on wharfs, and before fill began to be placed along the shoreline to extend the useable land area. (Photo credit: Excerpted from Alaska State Library – Historical Collections, [ASL-P87-0753](#), Winter & Pond, ca. 1896.)**

Adjacent to the Main Street Garage (a multi-level parkade) at the southeast end of Telephone Hill, on the northeast side of the bedrock ridge, the bedrock slope was cut to make room for the structure, with some rock-bolting also done to protect a residence on top of the ridge. The bedrock face at that location is regularly inspected and scaled if/as needed. The State Archives and Records Center, on the Willoughby Avenue side, is built into the ridge, with

an adjacent retaining structure just southeast of the back of the building. The State Office Building appears to have been built alongside and across the top of the ridge. Several other residences remain on top of the ridge. Numerous bedrock outcrops are present along Dixon Street in this area. Heading northwest along Dixon Street, Calhoun Avenue, Goldbelt Avenue, and Main Street into the Juneau Townsite and Chicken Ridge areas, retaining walls and buildings set into the slope are common, as well as bedrock outcrops.

Only a few historical landslides have been documented in this part of Juneau. These landslides are plotted on Figure 2 for information and comparison.

On September 7, 1923, the Juneau Daily Empire reported that a landslide “of about 100 feet occurred on the hill between Calhoun Avenue and Willoughby Avenue, at the foot of Dixon Street.” The slide had occurred early that morning due to the heavy rainfall of the preceding few days. The slide pushed a large unoccupied two-storey frame house off its foundations, moving it several feet. No damage was reported to the house, except that its “underpinnings” had been torn out. In addition, the slide also destroyed “part of the stairway leading from Calhoun Ave. to the Indian village” (The Alaska Daily Empire 1923). This landslide occurred in colluvium and appears to have been directly downslope of the fork at Dixon Street and Calhoun Avenue, in an area that still experiences periodic landslides, at the northwest corner of the Juneau Townsite in CBJ’s [Historic Neighborhoods](#) mapping.



**Figure 2 Excerpt of map of Downtown Juneau’s [Historic Neighborhoods](#) (CBJ 2022), showing selected historical landslide locations. The 1923 landslide is shown running downslope from Calhoun Avenue and the southeast end of Dixon Street to a little above the present-day Willoughby Avenue (red arrow). Landslides still occur at Calhoun Avenue, originating upslope of Dixon Street (green arrow).**

CBJ has reported that debris slides occur regularly at the fork between Dixon Street and Calhoun Avenue, between the northwest end of the retaining wall on Calhoun, and just downslope of the West 6<sup>th</sup> Street cul-de-sac. This location overlaps the southwest corner of Chicken Ridge, and the northwest corner of the Juneau Townsite in CBJ’s [Historic Neighborhoods](#) (CBJ 2022) mapping, and its location is shown in Figure 2. Rocks on the road have been reported after large storm events in at least the past two years. Trees or pieces of large woody debris are less frequent, occurring at roughly five-year intervals (email communications: July 20, 2021; A. Pierce, T. Camery, Q. Tracy, V. Roujanski, and R. Kors-Olthof). Google Street View suggests that debris appears to originate at a

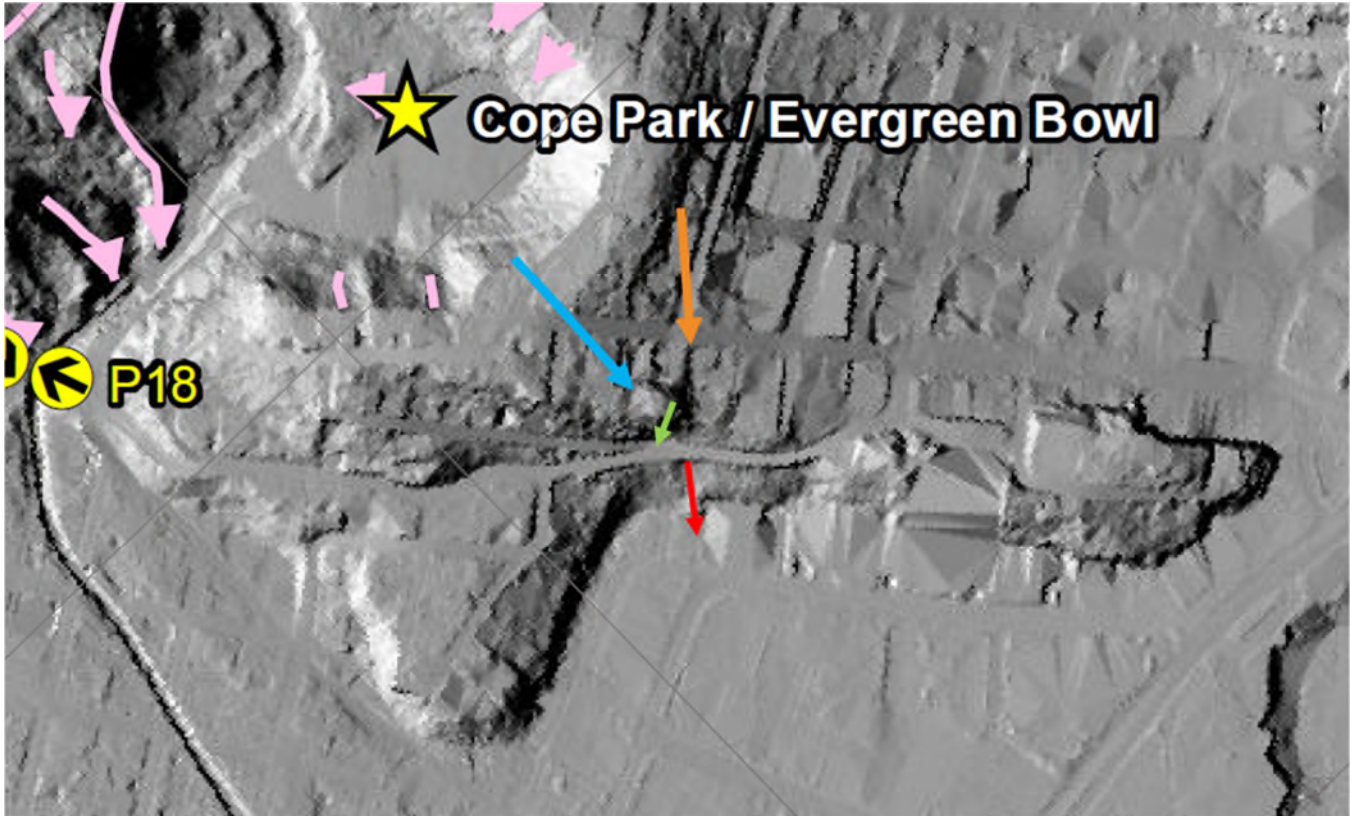
bedrock bluff partway upslope towards West 6<sup>th</sup> Street, and could consist of soil, rocks, trees, or other large woody debris, and other organic debris that typically lands on Dixon Street, requiring cleanup to restore road access, as seen in Figure 3. Part of the slope, between Dixon Street and Main Street upslope, is occupied by a building, but some debris seems to originate from the lower slope below the building too.



**Figure 3: Typical debris slide deposit at the fork between Dixon Street and Calhoun Avenue. In the left-hand photo, note the fork in the road at center-right, and the northwest end of the retaining wall at the right edge of the photo. The downslope edge of Calhoun at the railing is supported by another retaining wall. In the right-hand photo, note the presence of a steep cutslope into apparent weathered bedrock, with a thin veneer of colluvium. The debris from this landslide event extends an estimated 40 feet northwest of the end of the metal railing. The distance along the toe of slope along Dixon/Calhoun is about 110 feet between the northwest edge of the debris and the northwest corner of the upslope retaining wall. (Photo credits: CBJ, provided July 16, 2021.)**

There are a great many retaining walls visible on Dixon Street, Calhoun Avenue, Goldbelt Avenue, parts of Main Street, and connecting streets. Some of these retaining walls were apparently used to construct houses and associated landscaping to make more efficient use of the slope, but numerous retaining walls along roads appear to be necessary to create or maintain access to properties, or to reduce landsliding along steep slopes. In adjacent locations that lack retaining walls, many slope sections have either deciduous trees or grasses, suggesting that shallow debris slides might be fairly common. However, the specific location shown in Figure 3, and the landslide downslope in 1923 at almost this exact location, do suggest that there is something particular about this site that causes it to be exceptionally prone to slope failure. An excerpt of the LiDAR in this area provides a possible explanation (Figure 4).

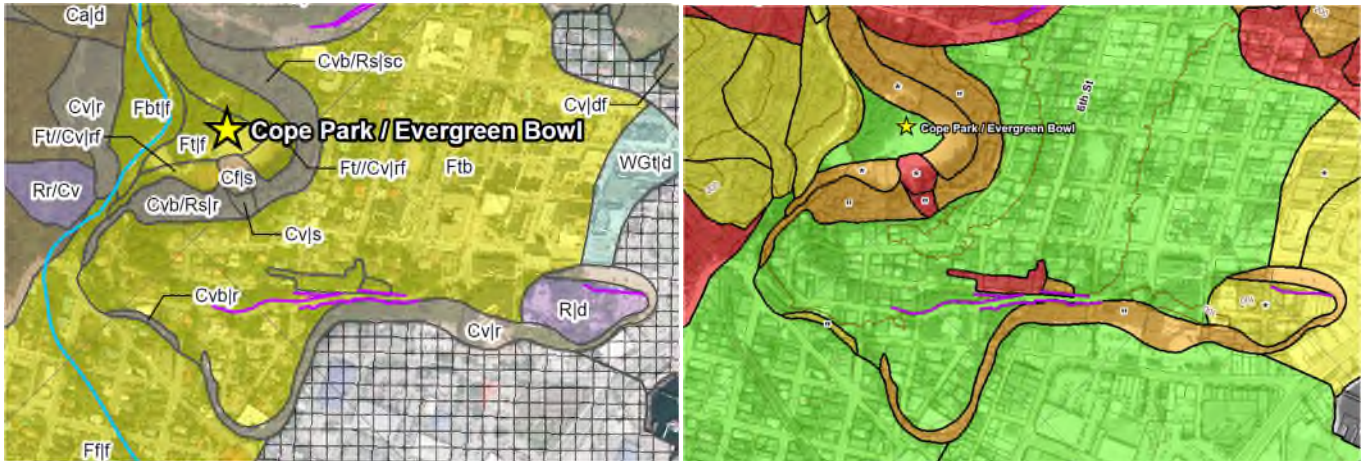




**Figure 4: Just off the southwest end of West 6<sup>th</sup> Street, there is a prominent bedrock knoll (blue arrow), and an adjacent gully (downslope of the orange arrow) that both potentially contribute to repeated debris slides at this location that regularly run out onto Dixon Street (green arrow). The gully is also directly in line with what appears to be a scar from the 1923 landslide that moved from Calhoun Avenue down to Willoughby Avenue (red arrow). See also Figure 2.**

### 3.2 Comparison of Map Excerpts

A summary of Tetra Tech’s mapping along Telephone Hill, Juneau Townsite, and Chicken Ridge is shown in Figure 5, with surficial geology on the left and landslide hazard designation mapping on the right. There is a clear correlation between the types and shapes of the surficial geology units and the landslide hazard designations at that location. In Figure 6, the colluvial areas are distinguished by the vegetation visible along slopes where it is difficult to construct housing or other structures.



**Figure 5: Excerpts from Figure 1.3b Surficial Geology (left) and Figures 1.6c Landslide Hazard Designation Mapping (right). Surficial geology corresponds closely to landslide hazard designations.**



**Figure 6: Excerpts from Figure 1.4b Slope Movement Features and Figure 1.6c Landslide Hazard Designation Mapping. On the left-hand image, the 1923 landslide is shown running downslope from Calhoun Avenue and the southeast end of Dixon Street to a little above the present-day Willoughby Avenue (red arrow). Landslides still occur at Calhoun Avenue, originating upslope of Dixon Street (green arrow). On the right-hand image, the hazard mapping from 1987 has been restored and updated above Calhoun Avenue, based on the debris slides reported by CBJ.**

Depending on the date of construction of the Calhoun Avenue upslope retaining wall, the area currently shown as *High* in Figure 6 could be downgraded to *Moderate*, but the area shown as *Severe* should remain as is due to the high frequency of debris slides at that location.

## 4.0 GASTINEAU AVENUE (SLOPES OF MT. ROBERTS)

### 4.1 Summary of Historical Landslides in Areas Mapped *High* or *Severe*

Much of the slope along Gastineau Avenue and South Franklin Street has a landslide hazard designation of *Severe*, due to being in the runout zone of numerous major debris flow paths. For debris slides that initiate within *Severe* zones upslope, the likelihood is very high that they will also run out in *Severe* zones downslope. As for debris slides

in areas designated as *High* that occur between those *Severe* zones upslope, e.g., on open slopes between debris flow gullies, these slides can be small enough that they will initiate and run out all within the same *High* zone. Depending on the location; however, some slides that initiate in the *High* zones could run out into the *Severe* zones.

It would be useful to determine whether any of the well-documented major landslides on the slopes of Mt. Roberts that initiated in a *High* zone, and ended up in *Severe*, also reaching structures along Gastineau Avenue or further downslope. For comparisons of less well-known landslides, side-by-side comparisons can help in this task, as further discussed in Section 4.2. Technical Memos #3, #6, and #7 provide additional information on specific findings on Mt. Roberts (Appendix C of the main report; Tetra Tech 2022c, 2022f, 2022g). Figures 7, 8, and 9 provide some side-by-side mapping comparisons for surficial geology, mass movement features, and gully erosion features compared to landslide hazard designations near the northwest end of Mt. Roberts. Figures 10, 11, and 12 provide the same side-by-side comparisons at the southeast end of the Study Area at Snowslide Creek. Details for two major landslides near the northwest end of Mt. Roberts are discussed below.

**Landslide of January 2, 1920:** Most of this landslide area is mapped in *Severe*, including the two houses that were destroyed above Gastineau Avenue (see Tetra Tech 2021a, Figure 1.6c, the first complete debris flow path from the right). Consider the report of overflowing water from the Alaska Juneau Gold Mining Company (AJGMC) flume in the time leading up to the landslide (The Alaska Daily Empire 1921). If that water came from Portal #1 (Figures 1a,1b in Technical Memo #7), does that mean it poured down the northwest side of the fill/spoil slope, triggering a slope failure in *High*? (See Figure 5 in Technical Memo #7.) It seems possible, mainly because AJGMC lost a court case in 1921 that contested whether or not AJGMC's leaky flume had contributed to that slide. On the other hand, AJGMC won three other court cases about the exact same landslide. A debris slide was not visible on the 1948 air photos on the southeast side of the 1920 gully; however, it is plausible that the scar from that event must have been fully revegetated by 1948, when the earliest set of air photos used in this project were taken (Figure 1.4b in the main report). In contrast, debris slide activity was mapped to the northwest of the 1920 debris flow gully on the 1977 air photos (suggesting that it occurred sometime after 1962, but before 1977). That debris slide area was located partly in *High* and partly in *Severe*, crossing several narrow hazard designation zones across its width (Figures 7 and 8). The toe of the debris slide area was about 100 feet in elevation above the cutline for the powerline above Gastineau Avenue.

**Landslides of November 22, 1936:** The tension crack reported below the AJGMC tramline (presumably within the fill/spoil slope) seems suspicious, suggesting initiation of the slope failure in *High*. However, the slide seems to have entered the runout in *Severe* along the southeast edge of the runout cone encompassing the former AJGMC office (for 1936B), and the adjacent slide (1936A) seems to have been entirely within *Severe* (Figures 1a, 1b in Technical Memo #7). Several debris slides were mapped in this area thereafter, apparently on the fill/spoil slope, in 1962, 1977 (confirming the 1971 air photo mosaic map from the State of Alaska, Department of Highways (ASL 2022)), and in 2013 and 2019 (Figure 8; Tetra Tech 2021a, Figure 1.4b). All but one of these later debris slides ran out to approximately the upper edge of the cleared powerline right-of-way, and the 2019 debris slide ran out to the lower edge of the right-of-way (Figure 8). Another debris slide or flow was mapped on the 1977 air photos, originating from upslope of the tramline, apparently flowing along the northwestern edge of the 1936B debris slide path, but running out well above the powerline right-of-way (Figure 8).

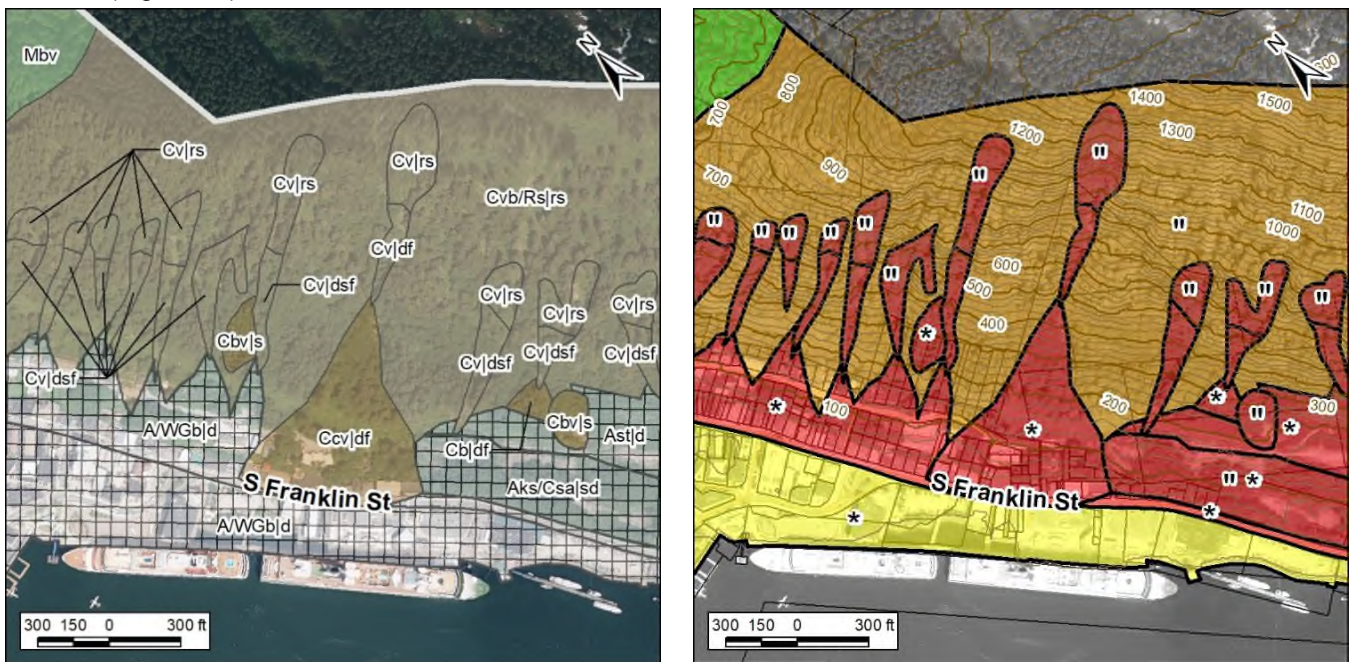
Landslide events that reach the lower slopes of Mt. Roberts tend to consist of debris flows or debris slides, and runouts are typically mapped in *Severe* on this slope. Those debris flows or debris slides could incorporate debris originating from areas mapped as *High* within the colluvium on the mid- to lower slopes, as noted above. The length of the slopes on Mt. Roberts means that there could be a few different types of landslide events between the top and bottom of the slope (Figures 7, 8, and 9 at the northwest end of Mt. Roberts; Figures 10, 11, and 12 at Snowslide Creek). Just as for Telephone Hill, and the adjacent Juneau Townsite and Chicken Ridge in terrain representing the same geological feature as Telephone Hill, wherever debris slide or debris flow processes are occurring now,

these are the kinds of mass movement processes that have been ongoing for decades and centuries, and they are expected to continue.

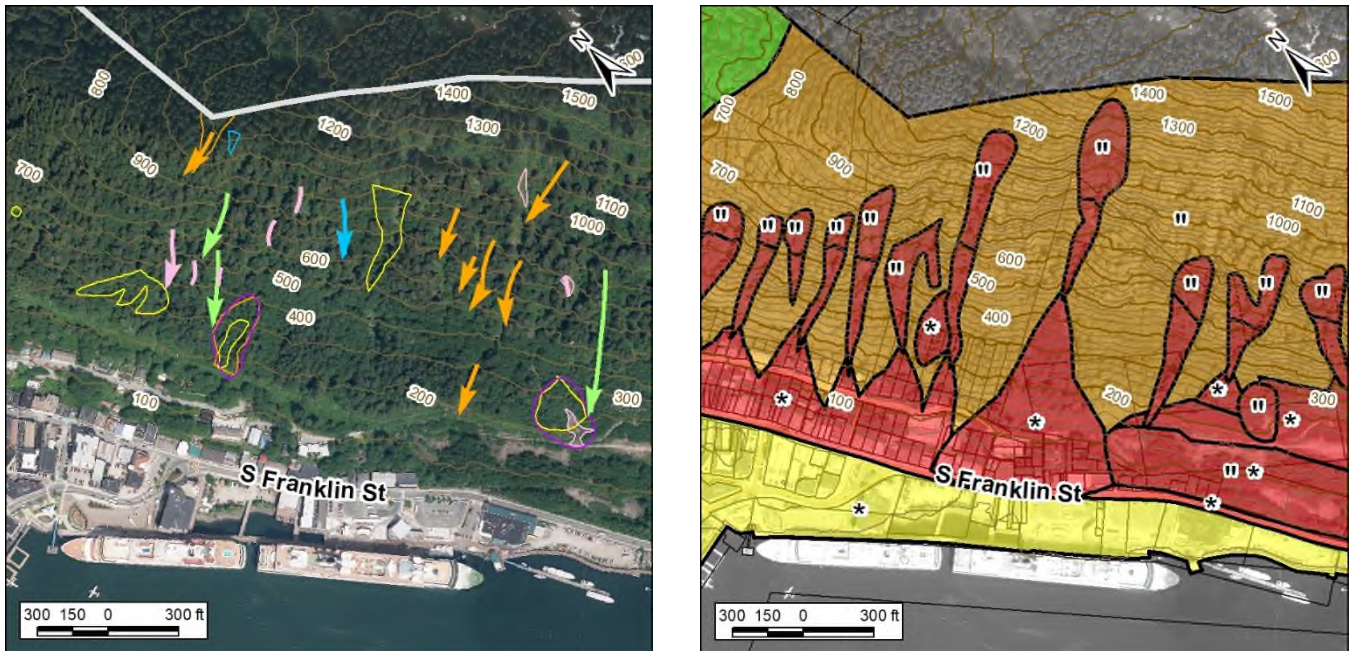
The processes described for Telephone Hill and the adjacent Juneau Townsite and Chicken Ridge are the same as those occurring on the slopes of Mt. Roberts, though the slope length is greater on Mt. Roberts, and although debris slides on open slopes can be similar in size to those above Telephone Hill and the adjacent areas, larger-scale events are possible, particularly for debris flows or debris slides within gullies (Figures 7, 8, 9, 11, and 12 below; Figures 1.4b, 1.5b, 1.4c, and 1.5c in Tetra Tech 2021a). However, many of the debris slides in *High* zones on Mt. Roberts terminate well above residential or commercial areas, and it is mainly where the debris slides coincide with *Severe* zones that they become more concerning.

## 4.2 Comparison of Map Excerpts

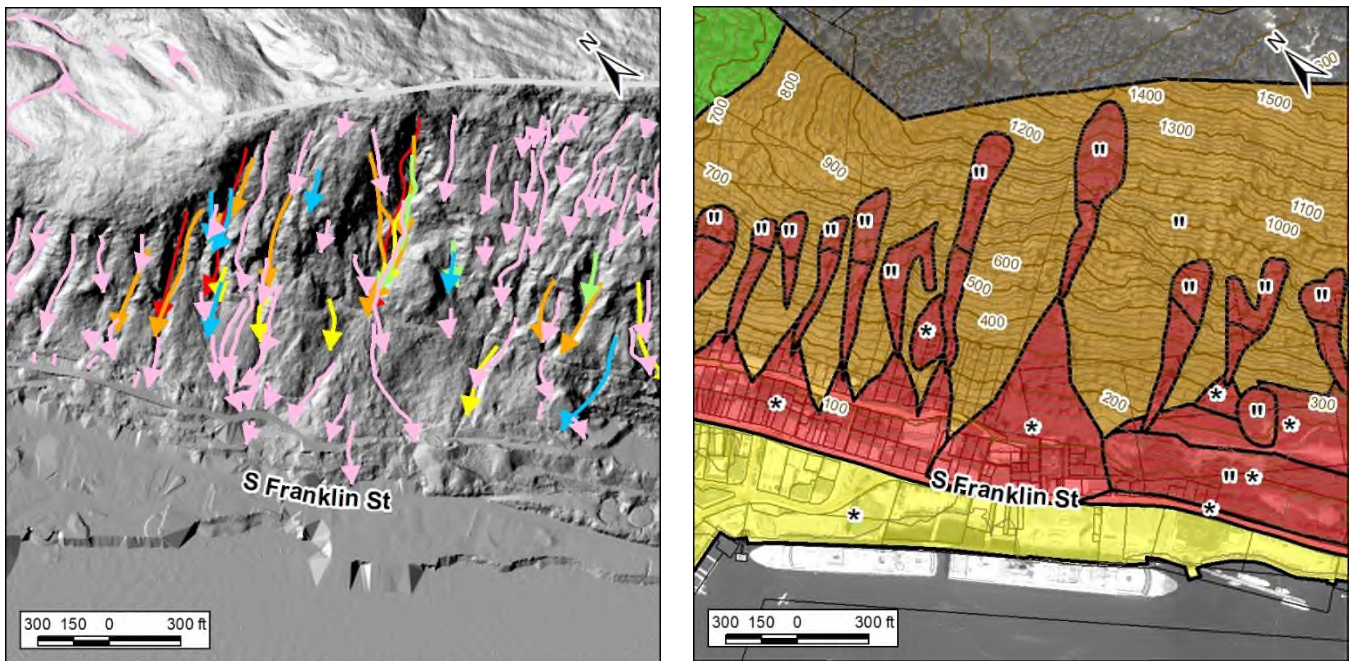
A summary of Tetra Tech’s mapping near the northwest end of Mt. Roberts is shown in Figure 7, with surficial geology on the left and landslide hazard designation mapping on the right. There is a clear correlation between the types and shapes of the surficial geology units and the landslide hazard designations at that location. The same correlations can be seen in the side-by-side comparisons of slope movement features (Figure 9) and gully erosion features (Figure 10).



**Figure 7: Excerpts from Figure 1.3b Surficial Geology (left) and Figures 1.6c and 1.6h Landslide Hazard Designation Mapping (right) near the northwest end of Mt. Roberts. Surficial geology corresponds closely to landslide hazard designations.**



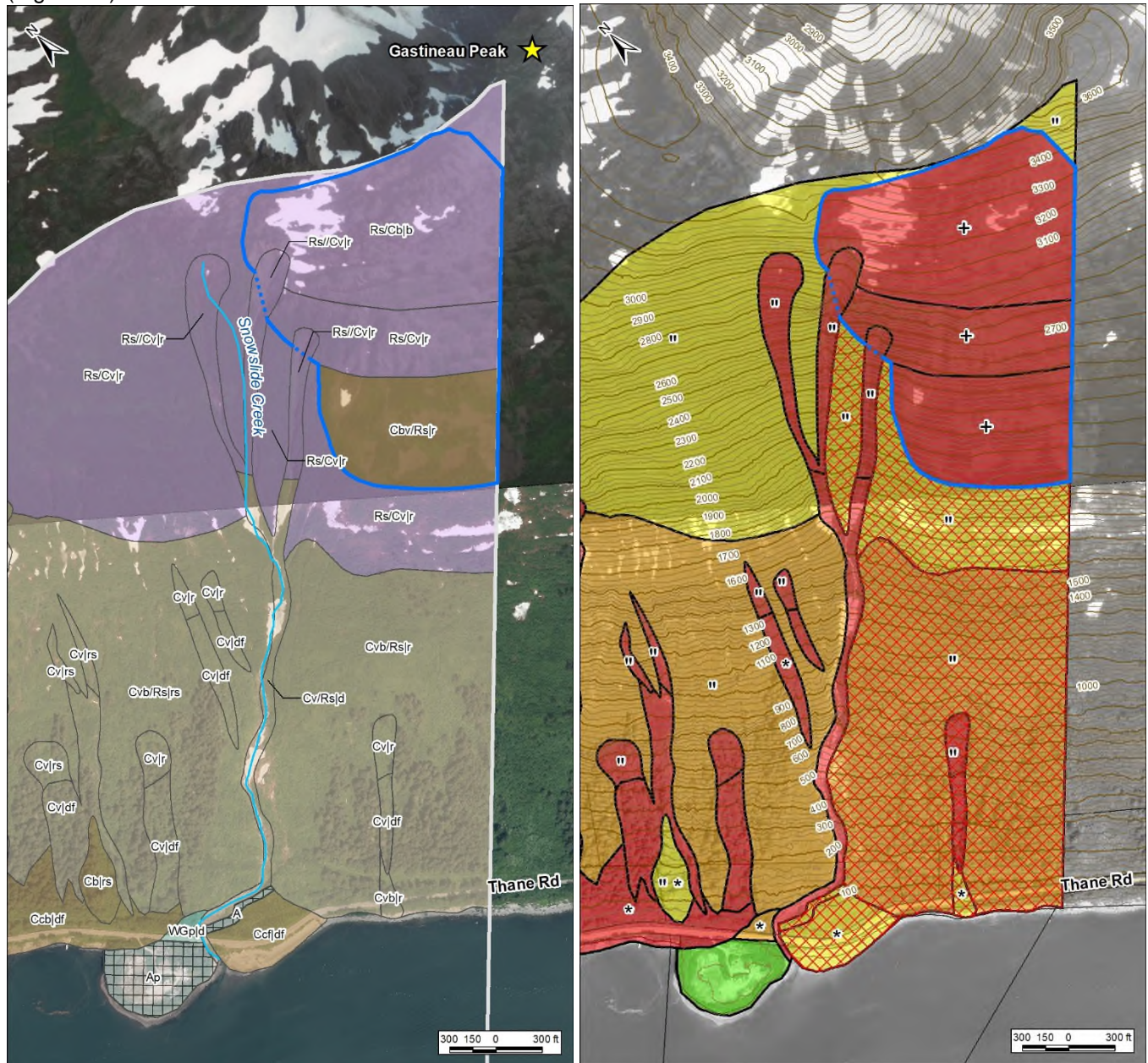
**Figure 8: Excerpts from Figure 1.4b Slope Movement Features (left) and Figures 1.6c and 1.6h Landslide Hazard Designation Mapping (right) near the northwest end of Mt. Roberts. Note outlines of several colours at center-left, indicating several years of landslide events at the same location, just upslope of the powerline right-of-way.**



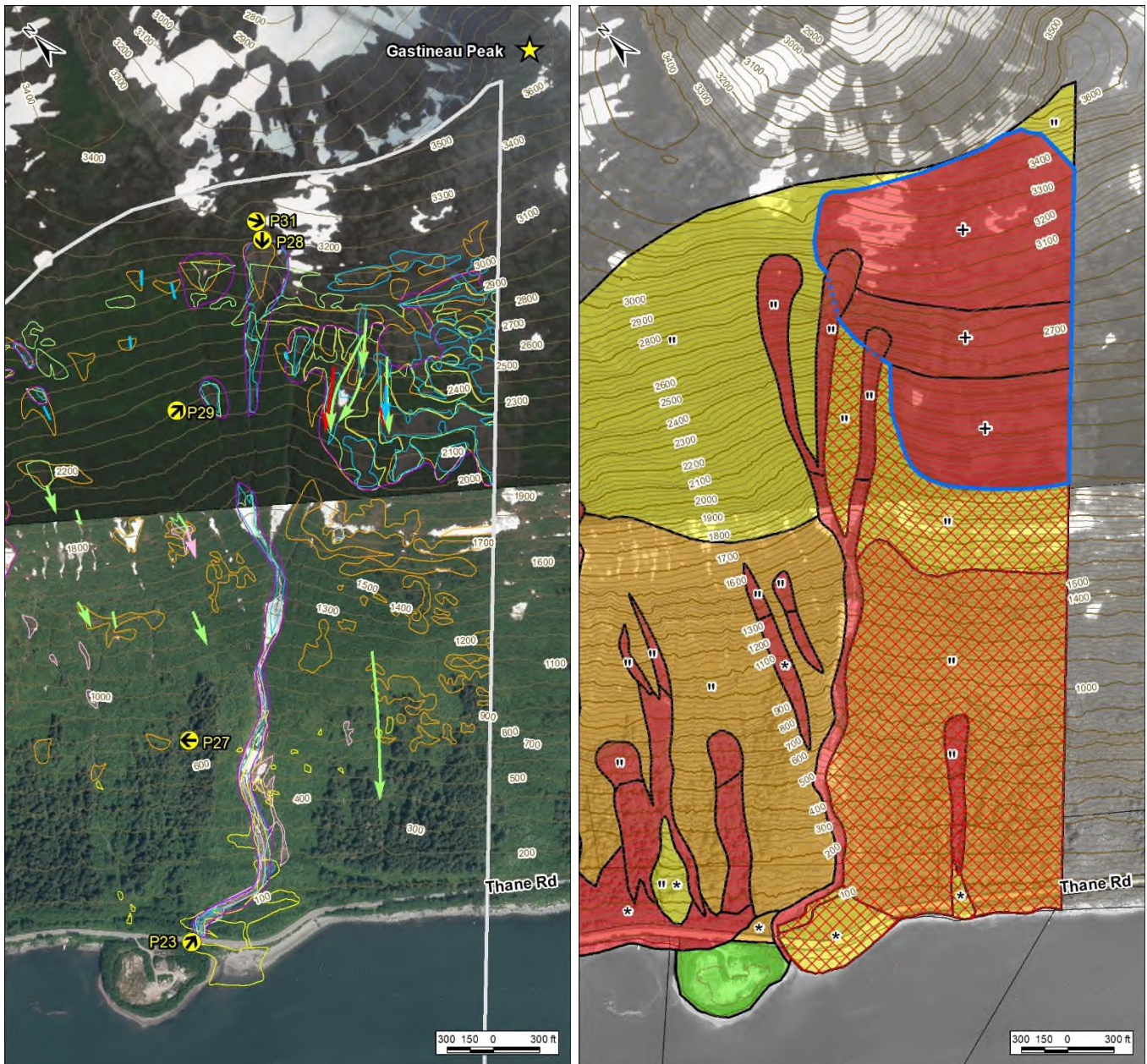
**Figure 9: Excerpts from Figure 1.5b Gully Erosion Features (left) and Figures 1.6c and 1.6h Landslide Hazard Designation Mapping (right) near the northwest end of Mt. Roberts. Multiple colours in gullies mean more activity than gullies that only have one colour.**

The same comparisons can be made further southeast along the slope, for example, for the terrain at Snowslide Creek, with surficial geology on the left and landslide hazard designation mapping on the right (Figure 10). Once again, there is a clear correlation between the shapes of the surficial geology units and the associated landslide

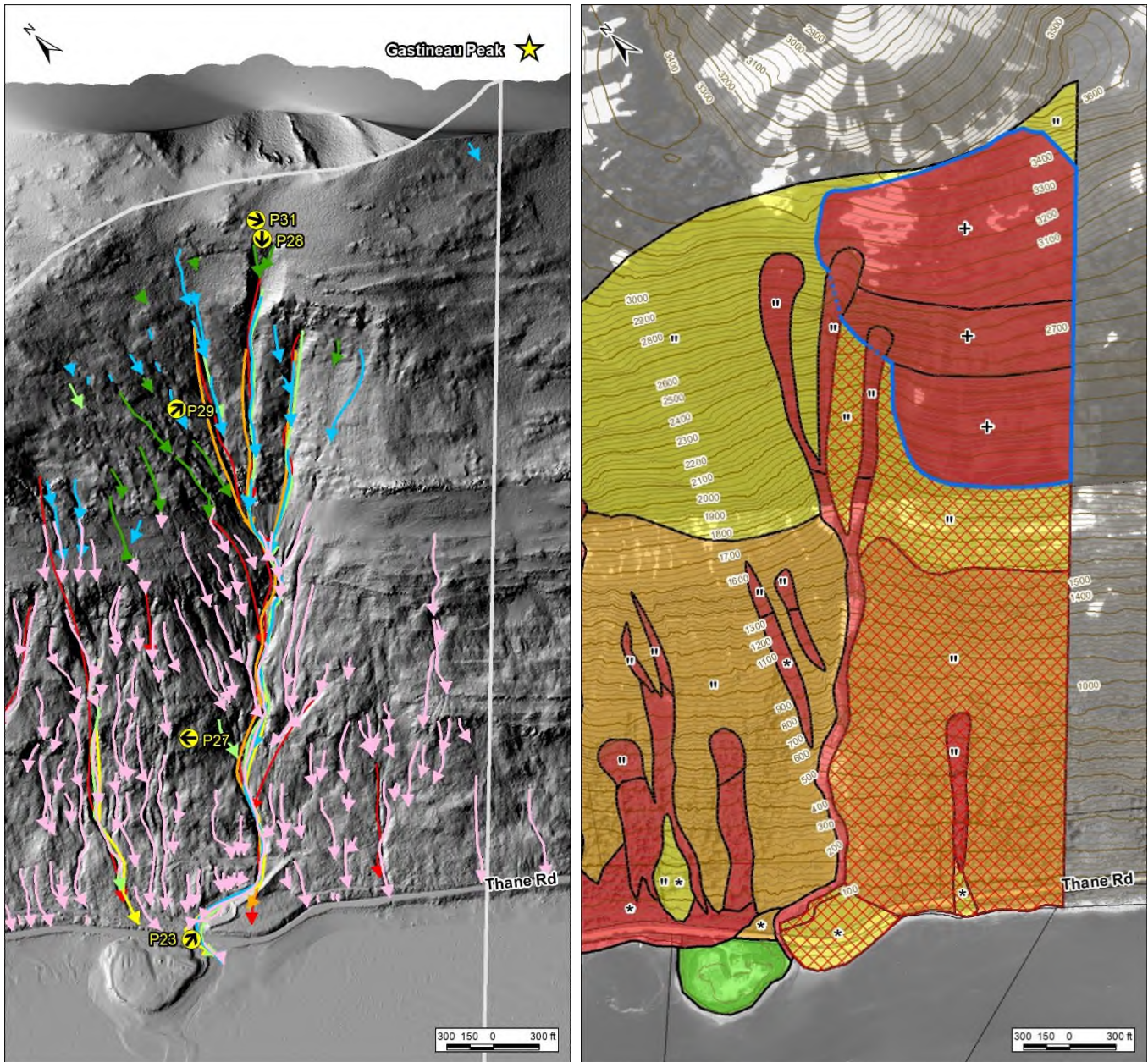
hazard designations, as well as the type of geology and the resulting hazard designation. The same correlations can be seen in the side-by-side comparisons of slope movement features (Figure 11) and gully erosion features (Figure 12).



**Figure 10: Excerpts from Figure 1.3c Surficial Geology (left) and Figures 1.6e and 1.6f Landslide Hazard Designation Mapping (right) at Snowslide Creek at the southeast end of the Study Area. Again, surficial geology corresponds closely to landslide hazard designations.**



**Figure 11: Excerpts from Figure 1.4c Slope Movement Features (left) and Figures 1.6e and 1.6f Landslide Hazard Designation Mapping (right) at Snowslide Creek at the southeast end of the Study Area. This comparison shows that many slope movement features, such as debris slides, are located on open slopes that typically have a *Moderate* or *High* landslide hazard designation. The exception in this part of the Study Area is the suspected deep-seated bedrock slide on the open slope at the top right of each image, with lots of slope movement features and a *Severe* landslide hazard designation. Slope movement features that take place within gullies contribute to a *Severe* landslide hazard designation, shown dramatically here at Snowslide Creek.**



**Figure 12: Excerpts from Figure 1.5c Gully Erosion Features (left) and Figures 1.6e and 1.6f Landslide Hazard Designation Mapping (right) at Snowslide Creek at the southeast end of the Study Area. In this comparison, the reason for the Severe landslide hazard designation at highly active gullies becomes clear. The more colours of arrows (representing different years of air photos in which erosion was observed), the more likely that a Severe rating is required. Note that minor gullies on otherwise open slopes do not elevate the rating for the open slopes, which are generally rated *High* for the lower slopes and *Moderate* for the upper slopes.**

The presence of major active gullies on Mt. Roberts shows the main difference between Mt. Roberts and Telephone Hill, where gullies are not so prevalent (or obvious). As shown by the landslide hazard designation mapping, Telephone Hill and nearby neighborhoods to the northwest generally have lower hazard ratings than Mt. Roberts.



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## 6.0 CLOSURE

We trust this technical memo meets your present requirements. If you have any questions or comments, please contact the undersigned.

Respectfully submitted,  
Tetra Tech Canada Inc.



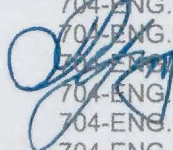
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- The Alaska Daily Empire, 1923. News article: "Landslide Takes Out Stairway Leading to Local Indian Village." The Alaska Daily Empire, p.2. Image provided by Alaska State Library Historical Collections and reproduced at the Library of Congress. [The Alaska daily empire. \[volume\], September 07, 1923, Page 2, Image 2](#). Access date: April 1, 2022.

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<b>To:</b>	Teri Camery (CBJ)	<b>Date:</b>	April 27, 2022
<b>c:</b>	Scott Ciambor (CBJ)	<b>Memo No.:</b>	6
<b>From:</b>	Rita Kors-Olthof, Vladislav Roujanski	<b>File:</b>	704-ENG.EARC03168-02A
<b>Subject:</b>	Severe Landslide Hazard Designations at Starr Hill and Gastineau Avenue Downtown Juneau Landslide and Avalanche Hazard Assessment		

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## 1.0 INTRODUCTION

Tetra Tech Canada Inc. (Tetra Tech) has prepared an Issued-for-Review (3<sup>rd</sup> Draft) Report, Downtown Juneau Landslide and Avalanche Assessment for the City and Borough of Juneau (CBJ), dated May 28, 2021 (Tetra Tech 2021); and participated in three Landslide and Avalanche Hazard Public Meetings that took place on July 21, August 10, and September 20, 2021.

Following CBJ's initial email request of July 27, 2021, Tetra Tech responded to comments and questions that arose from the July 21, 2021, Public Meeting with a series of three technical memos. These memos were Issued-for-Review to CBJ, along with an email providing supplemental information, and have since been updated (Tetra Tech 2022a, 2022b, 2022c).

CBJ has now requested a further series of memos to address additional landslide-related questions from the public, as well as a review of historical avalanche data to address further questions that arose following the August 10 and September 20, 2021, Public Meetings; as well as some follow-up questions from CBJ. The scope is as described in Tetra Tech proposal of December 9, 2021, with a few modifications as discussed during the kick-off meeting with CBJ on February 8, 2022. All the completed technical memos will be appended to the Final Draft Report.

This Technical Memo #6 provides some additional explanation of anticipated continued slope instabilities within the landslide hazard designations mapped as *Severe* on the slopes above Starr Hill and Gastineau Avenue.

## 2.0 SCOPE AND METHODS

The primary objective of this technical memo is to address the question, "The chutes mapped as *Severe* above Gastineau/Starr Hill scour down to bedrock over and over – is a bedrock failure anticipated, or just more flushing from small landslides?" Specific tasks included the following:

- Review completed landslide hazard mapping;
- Locate suitable photographs illustrating landslide hazards in the above-noted map areas, if/as needed;
- Prepare map excerpts, if/as needed;
- Refer to information presented previously in other technical memos, as applicable; and
- Prepare Technical Memo, providing descriptions and/or comparisons, as needed.

## 3.0 STARR HILL

### 3.1 General Considerations

The slope conditions around the Starr Hill subdivision were discussed in detail in Technical Memo #3 (Appendix C of the main report; Tetra Tech 2021d). Rockfalls and rockslides are most prevalent on the slopes above 6<sup>th</sup> Street, but there are also areas of rockfalls and rockslides above other areas of the subdivision, as described in Technical Memo #3.

### 3.2 Rockfalls and Rockslides

As noted in Technical Memo #3, Question #1, locations with numerous unstable rock cliffs and bluffs above 6<sup>th</sup> Street can be expected to continue experiencing rockfalls and rockslides. Swanston (1972) noted that, although the bedrock dips into Last Chance Basin (on the north side of Mt. Maria), cyclical freeze-thaw of water in the fractures and joints of the exposed bedrock, and water acting as a lubricant in the cracks, result in instabilities. The elevated level of slope movement activity on this slope, including several well-established slide paths below prominent bedrock bluffs and cliffs, requires the slopes below the cliffs to be designated as *Severe* hazard. Similar processes can be anticipated anywhere in those locations where bedrock outcrops are present. Depending on the structural orientation of the bedrock (e.g., dipping into the slope or out of the slope), the mass movement process at the outcrop may look more like rockfall (including toppling), or rockslides. Tetra Tech's field records include numerous photos of bedrock outcrops, cliffs, or bluffs, many of which have detached blocks, indicating the likelihood of future rockfall, rockslides, or toppling.

Once in motion, rocks might tend to bounce and roll (for example, where loose rocks can move independently and stop against trees, or structures, or other objects that block them or slow them down (e.g., above much of 6<sup>th</sup> Street), or they could fall or slide as a larger mass and end up in a large talus cone downslope (e.g., corner of 6<sup>th</sup> and Nelson Streets). These are the kinds of processes that have been ongoing since long before Swanston's observations and are expected to continue, as shown in the photos from Tetra Tech's recent fieldwork (Tetra Tech 2021a, 2021d).

Some of the slide paths above 6<sup>th</sup> Street appear to be smooth and open, suggesting that rockfall and/or rockslides are relatively frequent, scouring the area with each event, and vegetation cannot readily become re-established. In some cases, the very steep slopes could also reduce the rate of revegetation. In other locations, deciduous vegetation has become re-established, but rockfall continues.

Where debris accumulates in gullies, for example, from bedrock cliffs or bluffs upslope, and/or from debris slides within the gullies, the potential exists for that debris to eventually become part of a debris flow. Small debris flows tend to accumulate in wedges in gullies, until a combination of debris and extreme precipitation or rapid snowmelt results in much larger debris flow event that can scour out the gully. Also, the addition of more debris from ongoing failures upslope could potentially result in slope failures resulting from overloading of debris on the slope, especially if combined with heavy rainfall or a rapid snowmelt. See also the discussions about debris flows in Technical Memo #2, Question #9, and Technical Memo #3, Question #4 (Appendix C of the main report; Tetra Tech 2022b, 2022c).

See Technical Memo #3 for excerpts from the mapping and photos from the slopes around Starr Hill.

## 4.0 GASTINEAU AVENUE (SLOPES OF MT. ROBERTS)

### 4.1 General Considerations

Two general areas are considered along Mt. Roberts with respect to rockfall or rockslides:

- Past and probable future natural slope instabilities originating on natural terrain, and becoming incorporated into debris lobes on open slopes or in gullies (most of the length of the current Mt. Roberts Study Area); and
- The potential very large deep-seated bedrock slide southeast of Snowslide Creek.

In the context of the question to be answered from Section 2.0, only the first of these items can be addressed with the information currently available. The evaluation of natural slope instabilities is based on the slope observations made during the mapping project and is applicable to the entire slope of Mt. Roberts within the Study Area (Tetra Tech 2021a).

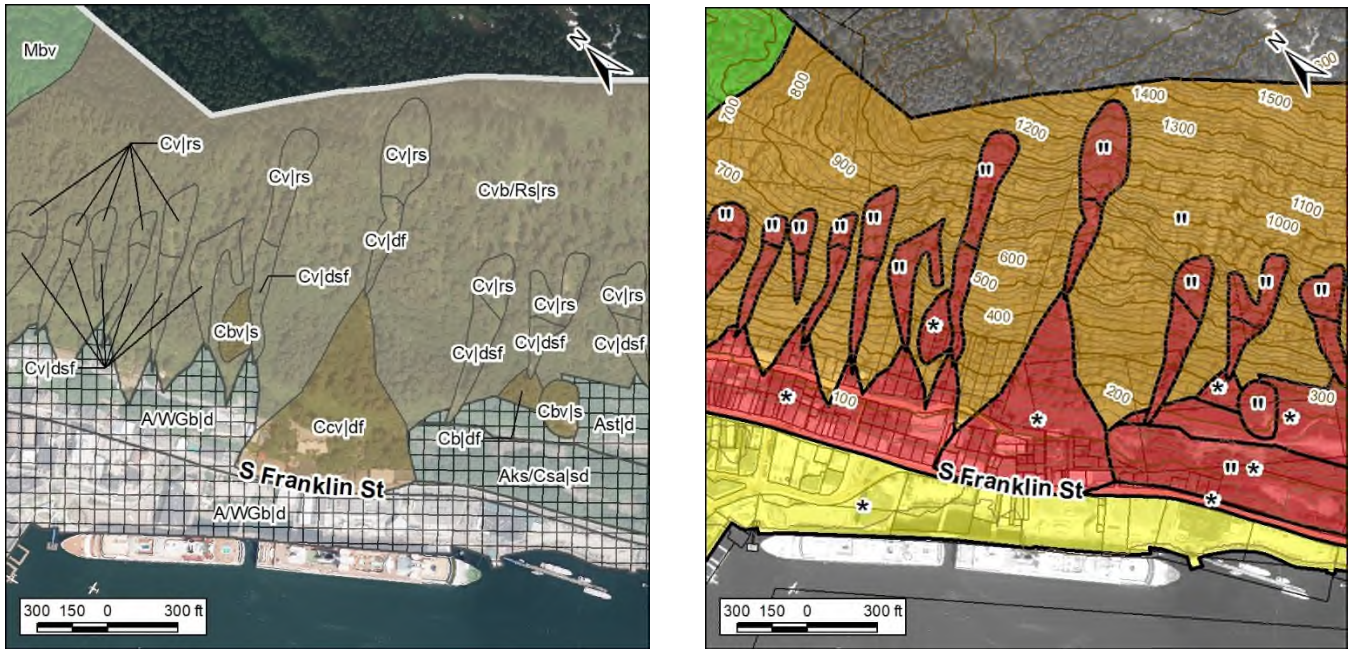
### 4.2 Rockfalls and Rockslides

In general, the same considerations as noted in Section 3.0 for Starr Hill also apply to Mt. Roberts. For example, the bedding planes of the bedrock on Mt. Roberts also dip into the slope, in this case, towards the northeast. However, the findings in Tetra Tech (2021a) suggested that although rockfall and rockslides (along with debris slides) could initiate in the upper portions of the slide paths on Mt. Roberts, landslide events that reach the lower slopes tend to consist of debris flows or debris slides. Those debris flows or debris slides could incorporate rock fragments originating from areas of bedrock outcrops within the colluvium on the mid to lower slopes or, in the case of Snowslide Creek, also from further upslope where the surficial materials consist mostly of bedrock. This does not mean that such events are less severe than rockfall or rockslide events, only that the length of the slope means that there could be a few different types of landslide events between the top and bottom of the slope. Just as for Starr Hill, wherever rockfall and rockslide processes are occurring now, these are the kinds of processes that have been ongoing for decades and centuries, and they are expected to continue.

The processes described for Starr Hill are the same as on Mt. Roberts, though the slope length is greater on Mt. Roberts, and although debris slides on open slopes are often similar in size to those above Starr Hill, larger events are possible, particularly for debris flows or debris slides within gullies (see Figures 1.4b, 1.4c, 1.5b, and 1.5c in Tetra Tech 2021a). In general, however, along the upper slopes of Mt. Roberts, where bedrock is more common at ground surface than colluvium (60% to 75% bedrock), or *much* more common than colluvium (80% to 95% bedrock), the slopes are considered more stable (rated *Moderate*) than the lower slopes that have more colluvium than bedrock (rated *High* or *Severe*). The only places where that rule-of thumb does not apply on Mt. Roberts is the potential very large deep-seated bedrock slide southeast of Snowslide Creek, and the three very large debris-flow initiation zones leading into Snowslide Creek itself (all rated *Severe*).

A summary of Tetra Tech's mapping near the northwest end of Mt. Roberts is shown in Figure 1, and the southeast end of the Study Area along Mt. Roberts is shown below in Figure 2, both with surficial geology on the left and landslide hazard designation mapping on the right. There is a clear correlation between the type and shapes of the surficial geology units and the landslide hazard designations.





**Figure 1: Excerpts from Figure 1.3b Surficial Geology (left) and Figures 1.6c and 1.6h Landslide Hazard Designation Mapping (right).**

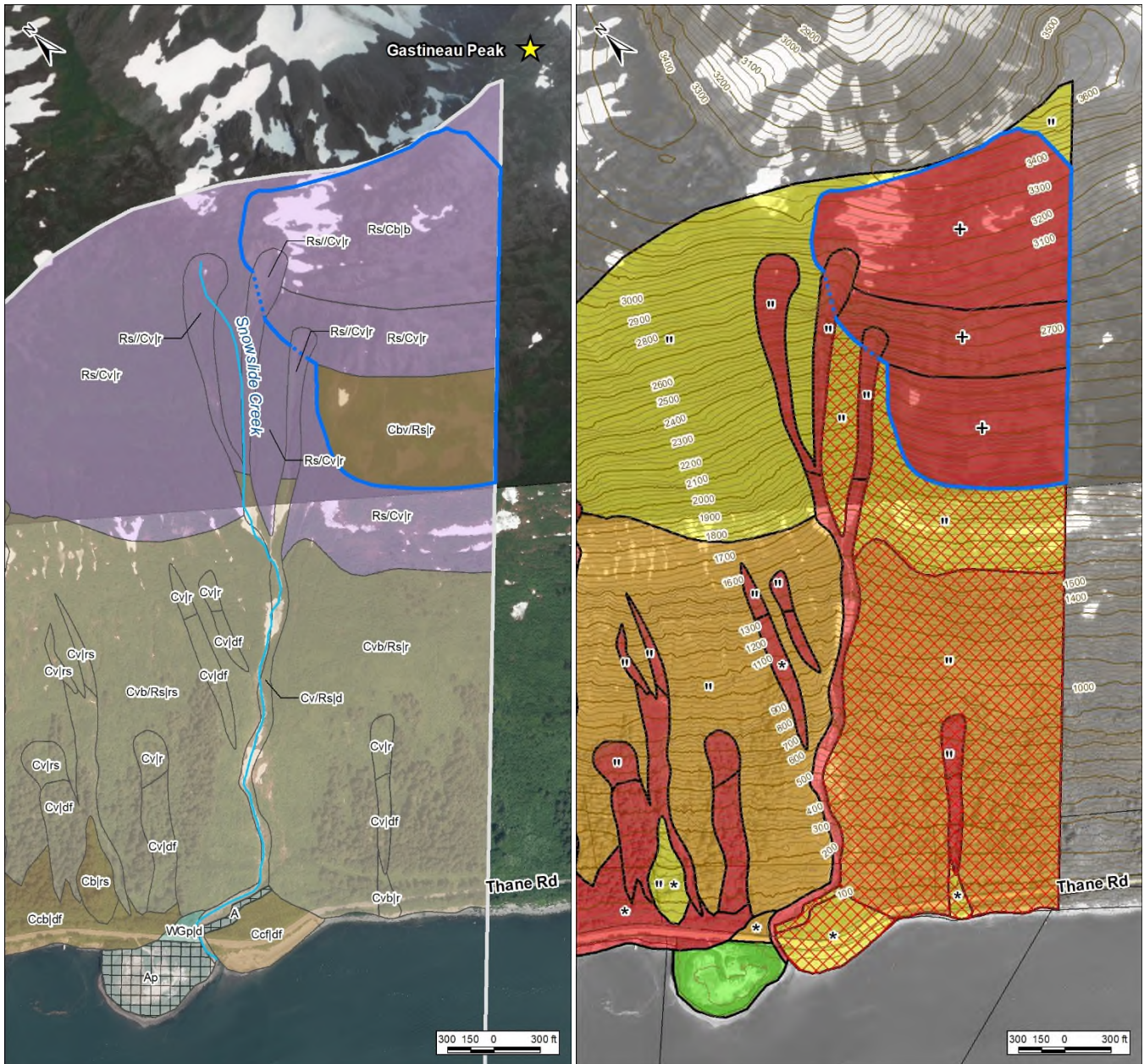


Figure 2: Excerpts from Figure 1.3c Surficial Geology (left) and Figures 1.6e and 1.6f Landslide Hazard Designation Mapping (right).

## 5.0 LIMITATIONS OF REPORT

This report and its contents are intended for the sole use of the City and Borough of Juneau and their agents. Tetra Tech Canada Inc. (Tetra Tech) does not accept any responsibility for the accuracy of any of the data, the analysis, or the recommendations contained or referenced in the report when the report is used or relied upon by any Party other than the City and Borough of Juneau and its agents, or for any Project other than the proposed development at the subject site. Any such unauthorized use of this report is at the sole risk of the user. Use of this document is subject to the Limitations on Use of this Document attached in the Appendix or Contractual Terms and Conditions executed by both parties.

## 6.0 CLOSURE

We trust this technical memo meets your present requirements. If you have any questions or comments, please contact the undersigned.

Respectfully submitted,  
Tetra Tech Canada Inc.



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Vladislav.Roujanski@tetrattech.com

/jf

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## REFERENCES

- Swanston, D. (1972). Mass Wasting Hazard Inventory and Land Use Control for the City and Borough of Juneau. Appendix II. Report to the City and Borough of Juneau, Alaska.
- Tetra Tech Canada Inc. (Tetra Tech), 2021. Downtown Juneau Landslide and Avalanche Assessment. Prepared for the City and Borough of Juneau (CBJ), Issued for Review (3<sup>rd</sup> Draft). Dated May 28, 2021. File Number: 704-ENG.EARC03168-01.
- Tetra Tech. (2022a). Technical Memo #1. Landslide Mapping Accuracy and Modelling, Downtown Juneau Landslide and Avalanche Hazard Assessment. Prepared for CBJ. Tetra Tech File: 704-ENG.EARC03168-02A. April 27, 2022.
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- Tetra Tech. (2022c). Technical Memo #3, Rev.1. Mapping Overview at Starr Hill Subdivision and Additional Information, Downtown Juneau Landslide and Avalanche Hazard Assessment. Prepared for CBJ. Tetra Tech File: 704-ENG.EARC03168-02A. April 27, 2022.

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## GEOTECHNICAL

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<b>From:</b>	Rita Kors-Olthof, Vladislav Roujanski	<b>File:</b>	704-ENG.EARC03168-02A
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## 1.0 INTRODUCTION

Tetra Tech Canada Inc. (Tetra Tech) has prepared an Issued-for-Review (3<sup>rd</sup> Draft) Report, Downtown Juneau Landslide and Avalanche Assessment for the City and Borough of Juneau (CBJ), dated May 28, 2021 (Tetra Tech 2021); and participated in three Landslide and Avalanche Hazard Public Meetings that took place on July 21, August 10, and September 20, 2021.

Following CBJ's initial email request of July 27, 2021, Tetra Tech responded to comments and questions that arose from the July 21, 2021, Public Meeting with a series of three technical memos. These memos were Issued-for-Review to CBJ, along with an email providing supplemental information, and have since been updated (Appendix C in the main report; Tetra Tech 2022a, 2022b, 2022c).

CBJ has now requested a further series of memos to address additional landslide-related questions, as well as a review of historical avalanche data, to address further questions that arose following the August 10 and September 20, 2021, Public Meetings; as well as some follow-up questions from CBJ. The scope is as described in Tetra Tech's proposal of December 9, 2021, with a few modifications as discussed during the kick-off meeting with CBJ on February 8, 2022. All the completed memos will be included in an appendix of the Final Draft Report.

This Technical Memo #7 provides some additional discussion about past and anticipated future slope instabilities potentially related to the past human activities, which shaped anthropogenic, i.e., human-modified terrain within the landslide hazard designations mapped as *Severe* on the slopes above Starr Hill and Gastineau Avenue.

## 2.0 SCOPE AND METHODS

The primary objective of this memo is to provide some additional background for responding to Question #14 in Technical Memo #3 (Appendix C in the main report; Tetra Tech 2022c). Since the potential influences of anthropogenic (human-modified) terrain can also affect the performance of these slopes, some additional interpretation and evaluation of these types of influences has also been considered. Specific tasks included the following:

- Review landslide hazard mapping;
- Locate suitable photographs illustrating landslide hazards in the above-noted map areas, if/as needed;
- Prepare map excerpts, if/as needed;
- Refer to information presented previously in other technical memos, as applicable; and



- Prepare Technical Memo, providing descriptions and/or comparisons, as needed.

## 3.0 STARR HILL

### 3.1 General Considerations

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The slope conditions around the Starr Hill subdivision were discussed in detail in Technical Memo #3 (Appendix C in the main report; Tetra Tech 2022c). Portions of these slopes are potentially affected by anthropogenic changes (human-made modifications). Above Starr Hill, such modifications mostly include the presence of trails, some of which are related to recreational hiking and access to the views from Mt. Roberts, and some of which are related to powerline alignments and/or possible former mining-related trails.

### 3.2 Effects of Human-Modified Terrain

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Technical Memo #3 provides an overview of the overall slope conditions above the Starr Hill subdivision, along with numerous photos and map excerpts. The main influence of human-modified terrain on the slopes around Starr Hill is the likelihood that earthworks along trails (and possibly the powerline alignments) might have blocked some of the natural swales and gullies that would ordinarily carry surface water runoff. Oversteepened cutslopes, or oversteepened or sidecast fillslopes, if present, also have the potential to result in, or contribute to, slope failures. The presence of human modifications on slopes also has implications for anticipating which slopes or slope sections might be more susceptible to landslides in the future, particularly if surface water drainage modifications or cuts and fills have disrupted the natural slope conditions. Figures 1 through 3 below present a few examples of former and/or active trails and other linear infrastructure on the slopes above Starr Hill. Conceivably, there could be still more trails not yet discovered on the imagery or historical photos.

As noted in Technical Memo #3, Question #12, surface water drainage along trails and other linear infrastructure, whether abandoned or actively in use, should be purposefully managed, so that the original natural drainage paths across these human-made features can be preserved or restored. For a detailed evaluation and recommendations for possible mitigations in human-modified terrain, a purpose-specific field investigation would be needed and is not part of the current scope. Recommendations for future investigation and evaluation are provided below in Section 5.0.



**Figure 1: Current Mt. Roberts hiking trail (red) and former hiking trail (pale gray). Switchbacks along new and old trails (orange arrows). Powerline cutlines are also visible (blue arrows). (Image credit: AllTrails 2021.)**



**Figure 2: Topographic map, showing the same trail alignments as the pale gray lines on Figure 1. Dotted trail above Nelson Street is the trail section officially no longer used. Note switchbacks. (Image credit: AllTrails 2022).**



**Figure 3: View of Mt. Maria and Mt. Roberts from Mt. Juneau, circa 1935. Note possible old forestry trail on the slopes above Starr Hill (red arrow) and the former Alaska Juneau Gold Mining Company (AJGMC) tramway (blue arrow). (Photo credit: Alaska State Library – Historical Collections, [ASL-P87-0542](#), Winter & Pond. ASL 2022a.)**

## 4.0 GASTINEAU AVENUE (SLOPES OF MT. ROBERTS)

### 4.1 General Considerations

Several past landslides on the slopes above Gastineau Avenue and South Franklin Avenue on Mt. Roberts were considered in Technical Memo #3, Question #14 (Tetra Tech 2021d). Of particular interest in that question was whether past landslides, such as those on deforested slopes in the vicinity of the siteworks of the former Alaska Juneau Gold Mining Company (AJGMC), should be considered representative of the potential for future landslides. The implication was that, because the logged slopes had revegetated, and mining-related blasting and water discharge were no longer taking place, landslides might not be as common as they once were on this slope. The answer to that question in Technical Memo #3 was that some of the landslides appeared to have been directly attributable to the mining-related siteworks or operations (e.g., the leaky flume apparently contributing to the January 2, 1920 landslide), or possibly suspicious (e.g., the tension crack seen below the flume in the November 22, 1936 landslide), but on the other hand, the cause of some of the landslides (e.g., the 1952 landslide) could *not* be directly attributed to the former mining infrastructure or operations.

What has not yet been directly considered for the Mt. Roberts slope is the possibility that the remnants of the tramway/railway grade, as well as roads, trails, or powerlines on Mt. Roberts might *still* potentially affect slope

stability on a large portion of the lower slope of Mt. Roberts, between the north flank at Starr Hill to at least as far south as the northwest end of Thane Road (Figure 1a, 1b), even though the mine has not been operating for decades (since 1944). The premise for the slopes on Mt. Roberts is the same as that for the slopes above Starr Hill. It is important to account for the following:

- Past and probable future natural slope instabilities originating on natural terrain, not specifically modified or influenced by human activities, addressed in Technical Memos #3 and #6 for this slope (Appendix C in the main report; Tetra Tech 2022c, 2022f); and
- Past and potential future slope instabilities in anthropogenic (human-modified) terrain, including areas of previous logging, old roads, trails, powerlines, and tramway/railway grades (this memo).

The evaluation of natural slope instabilities is based on the slope observations made during the mapping project and is applicable to the entire slope of Mt. Roberts within the Study Area (see main report).

At this time, only a preliminary evaluation of the effects of human-modified terrain is possible, based on the slope observations made during the mapping project, a subsequent LiDAR data review and air photo 3D-analysis in PurVIEW, and a review of historical photos and records from the Alaska State Archives – Historical Collections (2022a through 2022e), documenting a range of mass movement events on this slope. For a detailed evaluation and recommendations for possible mitigations in human-modified terrain, a purpose-specific field investigation would be needed and is not part of the current scope. Recommendations for future investigation and evaluation are provided below in Section 5.0. Some of the observed effects of human-induced terrain disturbance are described in Section 4.2.

## 4.2 Effects of Human-Modified Terrain

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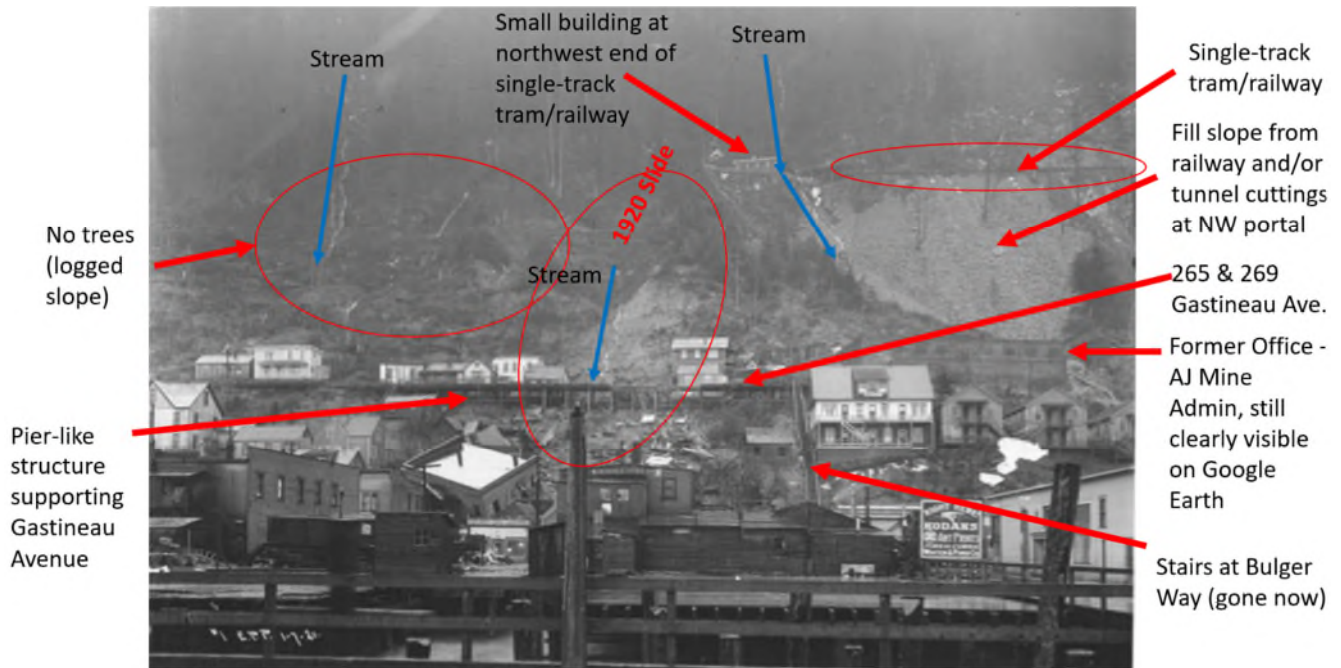
Some of the potential and documented human-induced slope instabilities were discussed in Technical Memo #3 (Appendix C in the main report; Tetra Tech 2022c). Subsequent desktop evaluation on the slope along Gastineau Avenue has revealed some additional useful information, which has been used to update Question/Comment #14 in Technical Memo #3, and is discussed in detail here.

Tetra Tech used numerous historical photos from the Alaska State Archives – Historical Collections (2022a through 2022e), maps, plans, and the LiDAR mapping for the landmarking of several major landslides that took place on the southeast (Gastineau Channel) side of Mt. Roberts, and previously described in Technical Memo #3, Question #14 (Appendix C in the main report; Tetra Tech 2022c). In one case, for the January 2, 1920 landslide, it was possible to directly compare a documented “before-and-after” set of photos (Figures 4 and 5), with confirmation of most of the structures available from the 1914 survey plans. In other cases, such as the November 22, 1936, landslide, for which photos of the slope had uncertain dates, a timeline of photos and maps was developed based on structures or slope features that were present or absent.

Figures 4 through 7 below present a few examples of former and/or active linear infrastructure, such as old roads, trails, powerlines, and tramway/railway grades, on the slopes above Gastineau Avenue on Mt. Roberts where landslides have occurred. Figure 1a, 1b, attached, provides some additional interpretation of the current slope conditions, the locations of some of the old mining infrastructure, and the locations of a few of the major slides over the past century. Conceivably, there could be more such features on the slope that have not yet been discovered on the imagery or historical photos. To reduce the likelihood of unexpected contributions from infrastructure to landslide occurrences or severity, more information could be collected on those features, and a decision-making process implemented to decide what to do about them, if anything.



**Figure 4: This “before” photo is from after 1914 but before 1920, and possibly from the summer of 1919. The stairway at center-right is Bulger Way. The pier-like structure is Gastineau Avenue. Decker Way (a stairway) is located just beyond the left edge of the photo. Note the numerous structures between Decker Way and Bulger Way before the landslide. Just up and left of the smoking-chimney building is the tenement building that tipped and rotated clockwise during the landslide (see Figure 5). Top right is the former Alaska Juneau Gold Mining Company (AJGMC) office building. (Photo credit: Alaska State Library – Historical Collections, PCA0154-295, Snow Family Photograph Collection. ASL 2022b.)**



**Figure 5: Compare to Figure 4 and note numerous missing, destroyed and/or shifted structures. Slope right of small building failed in November 22, 1936 landslide. (Photo credit: Alaska State Library – Historical Collections, [ASL-P87-1223](#), Winter & Pond, January 7, 1920; cropped to fit page, markups from CBJ and Tetra Tech. ASL 2022c.)**



**Figure 6: Two apparent debris paths from November 22, 1936, landslide (red arrows, see also Figure 1a, 1b attached). Structure at center-left was the Juneau Cold Storage Company building, later replaced. Debris ran up against the S. Franklin St. side of Juneau Cold Storage, moved the Madsen Building downslope, destroyed several buildings, and killed 15 people. (Photo credit: Alaska State Library – Historical Collections, [ASL-P134-312-4](#), date uncertain, cropped to fit. ASL 2022d.)**



Figure 7: Excerpt from 1971 air photo mosaic showing a reactivated slope failure at the site of the 1936 landslide (red arrow). Since the 1968 air photo mosaic showed little exposed soil at this location, this slope might continue to slough and ravel periodically over time, confirmed by Tetra Tech’s mapping of mass movement features (Tetra Tech 2021a). (Image credit: Alaska State Library – Historical Collections, [ASL- Map Case Juneau 1971](#), ALS 2022e.)

## 5.0 RECOMMENDATIONS FOR FUTURE WORK

A forestry road-deactivation format for slope review and the preparation of recommendations for proposed mitigations could be considered for future work.

The intent of the work would be to mitigate or reduce the potential for damage resulting from slope instabilities that are attributable to abandoned or active infrastructure on the slope, especially linear infrastructure that tends to alter surface water drainage. It might not be possible to prevent all infrastructure-related slope instabilities but could reduce the likelihood that the infrastructure triggers slope instabilities or that it makes the effects of natural slope instabilities worse.

## 6.0 LIMITATIONS OF REPORT

This report and its contents are intended for the sole use of City and Borough of Juneau and their agents. Tetra Tech Canada Inc. (Tetra Tech) does not accept any responsibility for the accuracy of any of the data, the analysis, or the recommendations contained or referenced in the report when the report is used or relied upon by any Party other than City and Borough of Juneau and their agents, or for any Project other than the proposed development at the subject site. Any such unauthorized use of this report is at the sole risk of the user. Use of this document is subject to the Limitations on Use of this Document attached in the Appendix or Contractual Terms and Conditions executed by both parties.



## 7.0 CLOSURE

We trust this technical memo meets your present requirements. If you have any questions or comments, please contact the undersigned.

Respectfully submitted,  
Tetra Tech Canada Inc.



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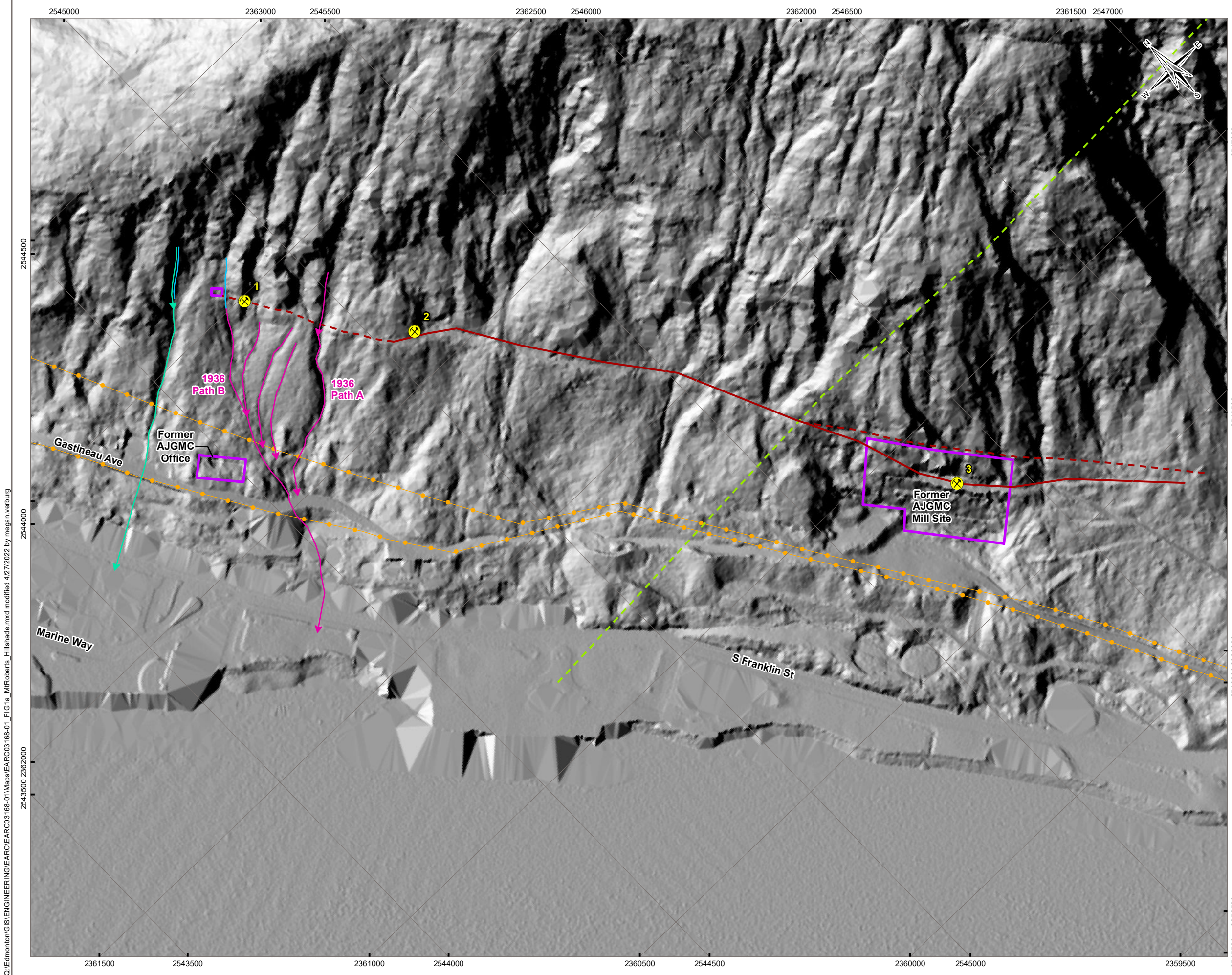
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/jf

Enclosure:      Limitations on Use of this Document  
                         Figures 1a, 1b

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### LEGEND

- Mine Portal
- 1** Interpreted from the 1948 air photos and 1916 drawing from Alaska-Juneau Gold Mining Co. (see References)
- 2** Interpreted from the 1948 air photos and 1916 drawing from Alaska-Juneau Gold Mining Co. (see References), and confirmed by Tetra Tech in the field
- 3** Interpreted from Juneau historical photos (Alaska State Library) and confirmed by Tetra Tech in the field
- 1920 Landslide - Interpreted from the 1948 air photos, historical photos (Collections from Alaska State Library), news reports, summary from Swanston (1972), and geologic mapping from Miller (1975)
- 1936 Landslide - Interpreted from the 1948 air photos, historical photos (Collections from Alaska State Library), summary from Swanston (1972), and geologic mapping from Miller (1975)
- Tramway/Railway Grade - Interpreted from the 1948 air photos and confirmed by Tetra Tech in the field
- Tramway/Railway Grade - Interpreted from the 1948 air photos and Juneau historical photos (Alaska State Library)
- Stream
- Gondola
- Power Line
- Building Footprint

### NOTES

Base data source:  
Hillshade derived from 2013 LiDAR data provided by CBJ.

STATUS  
ISSUED FOR USE

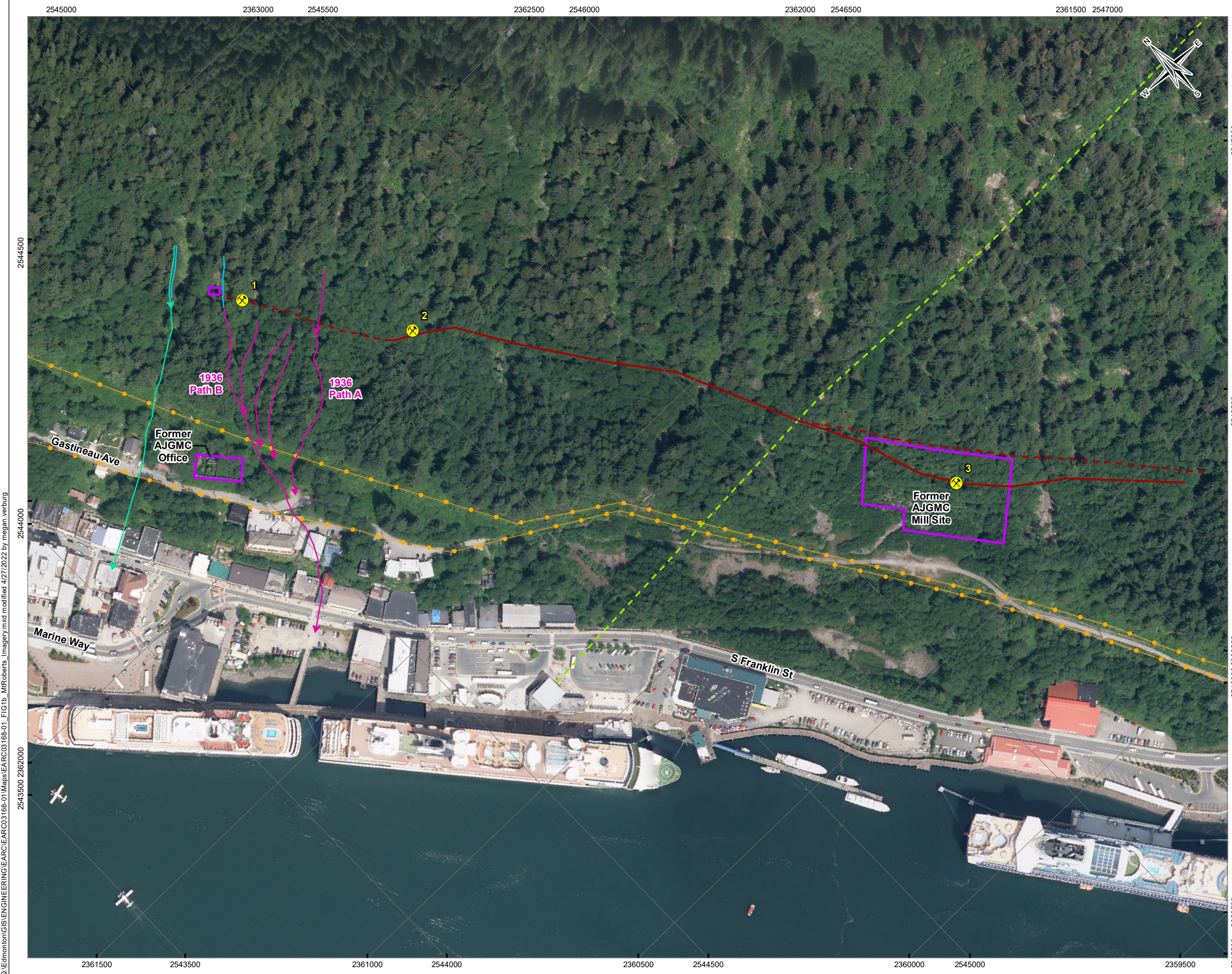
## JUNEAU LANDSLIDE AND AVALANCHE ASSESSMENT

### Locations of Infrastructure and Landslides on Mt. Roberts (LiDAR)

<b>PROJECTION</b> State Plane Alaska Zone 1 5001		<b>DATUM</b> NAD83		<b>CLIENT</b> 
Scale: 1:3,000				
<b>FILE NO.</b> EARC03168-01_FIG1a_MtRoberts_Hillshade.mxd				
<b>OFFICE</b> TL-EDM	<b>DWN</b> SL	<b>CKD</b> BB	<b>APVD</b> SM/ VER	<b>REV</b> 0
<b>DATE</b> April 27, 2022	<b>PROJECT NO.</b> ENG.EARC03168-01			

Figure 1a

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**LEGEND**

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- 2** Interpreted from the 1948 air photos and 1916 drawing from Alaska-Juneau Gold Mining Co. (see References), and confirmed by Tetra Tech in the field
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- Tramway/Railway Grade - Interpreted from the 1948 air photos and Juneau historical photos (Alaska State Library)
- Stream
- Gondola
- Power Line
- Building Footprint

**NOTES**  
 Base data source:  
 Primary imagery provided by CBJ, 2013.

STATUS  
 ISSUED FOR USE

**JUNEAU LANDSLIDE AND AVALANCHE ASSESSMENT**

**Locations of Infrastructure and Landslides on Mt. Roberts (Air Photos)**

<b>PROJECTION</b> State Plane Alaska Zone 1 5001		<b>DATUM</b> NAD83		<b>CLIENT</b> CITY AND BOROUGH OF JUNEAU ALASKA CAPITAL CITY
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<b>FILE NO.</b> EARC03168-01_FIG1b_MtRoberts_Imagery.mxd				
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<b>DATE</b> April 27, 2022	<b>PROJECT NO.</b> ENG.EARC03168-01			



**Figure 1b**

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### 1.4 DISCLOSURE OF INFORMATION BY CLIENT

The Client acknowledges that it has fully cooperated with TETRA TECH with respect to the provision of all available information on the past, present, and proposed conditions on the site, including historical information respecting the use of the site. The Client further acknowledges that in order for TETRA TECH to properly provide the services contracted for in the Contract, TETRA TECH has relied upon the Client with respect to both the full disclosure and accuracy of any such information.

### 1.5 INFORMATION PROVIDED TO TETRA TECH BY OTHERS

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While TETRA TECH endeavours to verify the accuracy of such information, TETRA TECH accepts no responsibility for the accuracy or the reliability of such information even where inaccurate or unreliable information impacts any recommendations, design or other deliverables and causes the Client or an Authorized Party loss or damage.

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This Professional Document is based solely on the conditions presented and the data available to TETRA TECH at the time the data were collected in the field or gathered from available databases.

The Client, and any Authorized Party, acknowledges that the Professional Document is based on limited data and that the conclusions, opinions, and recommendations contained in the Professional Document are the result of the application of professional judgment to such limited data.

The Professional Document is not applicable to any other sites, nor should it be relied upon for types of development other than those to which it refers. Any variation from the site conditions present, or variation in assumed conditions which might form the basis of design or recommendations as outlined in this report, at or on the development proposed as of the date of the Professional Document requires a supplementary investigation and assessment.

TETRA TECH is neither qualified to, nor is it making, any recommendations with respect to the purchase, sale, investment or development of the property, the decisions on which are the sole responsibility of the Client.

## 1.7 ENVIRONMENTAL AND REGULATORY ISSUES

Unless stipulated in the report, TETRA TECH has not been retained to investigate, address or consider and has not investigated, addressed or considered any environmental or regulatory issues associated with development on the subject site.

## 1.8 NATURE AND EXACTNESS OF SOIL AND ROCK DESCRIPTIONS

Classification and identification of soils and rocks are based upon commonly accepted systems and methods employed in professional geotechnical practice. This report contains descriptions of the systems and methods used. Where deviations from the system or method prevail, they are specifically mentioned.

Classification and identification of geological units are judgmental in nature as to both type and condition. TETRA TECH does not warrant conditions represented herein as exact, but infers accuracy only to the extent that is common in practice.

Where subsurface conditions encountered during development are different from those described in this report, qualified geotechnical personnel should revisit the site and review recommendations in light of the actual conditions encountered.

## 1.9 LOGS OF TESTHOLES

The testhole logs are a compilation of conditions and classification of soils and rocks as obtained from field observations and laboratory testing of selected samples. Soil and rock zones have been interpreted. Change from one geological zone to the other, indicated on the logs as a distinct line, can be, in fact, transitional. The extent of transition is interpretive. Any circumstance which requires precise definition of soil or rock zone transition elevations may require further investigation and review.

## 1.10 STRATIGRAPHIC AND GEOLOGICAL INFORMATION

The stratigraphic and geological information indicated on drawings contained in this report are inferred from logs of test holes and/or soil/rock exposures. Stratigraphy is known only at the locations of the test hole or exposure. Actual geology and stratigraphy between test holes and/or exposures may vary from that shown on these drawings. Natural variations in geological conditions are inherent and are a function of the historic environment. TETRA TECH does not represent the conditions illustrated as exact but recognizes that variations will exist. Where knowledge of more precise locations of geological units is necessary, additional investigation and review may be necessary.

## 1.11 PROTECTION OF EXPOSED GROUND

Excavation and construction operations expose geological materials to climatic elements (freeze/thaw, wet/dry) and/or mechanical disturbance which can cause severe deterioration. Unless otherwise specifically indicated in this report, the walls and floors of excavations must be protected from the elements, particularly moisture, desiccation, frost action and construction traffic.

## 1.12 SUPPORT OF ADJACENT GROUND AND STRUCTURES

Unless otherwise specifically advised, support of ground and structures adjacent to the anticipated construction and preservation of adjacent ground and structures from the adverse impact of construction activity is required.

## 1.13 INFLUENCE OF CONSTRUCTION ACTIVITY

There is a direct correlation between construction activity and structural performance of adjacent buildings and other installations. The influence of all anticipated construction activities should be considered by the contractor, owner, architect and prime engineer in consultation with a geotechnical engineer when the final design and construction techniques are known.

## 1.14 OBSERVATIONS DURING CONSTRUCTION

Because of the nature of geological deposits, the judgmental nature of geotechnical engineering, as well as the potential of adverse circumstances arising from construction activity, observations during site preparation, excavation and construction should be carried out by a geotechnical engineer. These observations may then serve as the basis for confirmation and/or alteration of geotechnical recommendations or design guidelines presented herein.

## 1.15 DRAINAGE SYSTEMS

Where temporary or permanent drainage systems are installed within or around a structure, the systems which will be installed must protect the structure from loss of ground due to internal erosion and must be designed so as to assure continued performance of the drains. Specific design detail of such systems should be developed or reviewed by the geotechnical engineer. Unless otherwise specified, it is a condition of this report that effective temporary and permanent drainage systems are required and that they must be considered in relation to project purpose and function.

## 1.16 BEARING CAPACITY

Design bearing capacities, loads and allowable stresses quoted in this report relate to a specific soil or rock type and condition. Construction activity and environmental circumstances can materially change the condition of soil or rock. The elevation at which a soil or rock type occurs is variable. It is a requirement of this report that structural elements be founded in and/or upon geological materials of the type and in the condition assumed. Sufficient observations should be made by qualified geotechnical personnel during construction to assure that the soil and/or rock conditions assumed in this report in fact exist at the site.

## 1.17 SAMPLES

TETRA TECH will retain all soil and rock samples for 30 days after this report is issued. Further storage or transfer of samples can be made at the Client's expense upon written request, otherwise samples will be discarded.